



Prolam[®]
I-Joists

Prolam I-Joists Design Guide

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Scope of this Publication

This Design Guide and Load Tables assists in the selection of SmartJoists for most of the common structural arrangements met in domestic construction. The Prolam Online computer software, in conjunction with this manual, provides an unparalleled level of design capacity for engineered timber products.

While specific details are given on suitable methods of developing lateral restraint, the methods of providing adequate support, adequate anchorage against wind uplift and overall structural stability are outside the scope of this publication.

Substitution of other Products

All load tables in this document are designed using ingrade tested properties for SmartJoists as manufactured by Pacific Woodtech Corporation of Washington State, USA. Other manufacturers I-joists may have different properties and, therefore, cannot be designed using these span tables.

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Certification

As a professional engineer, qualified and experienced in timber engineering, I certify that the use of the SmartJoist members as shown in these tables, and installed in accordance with the provisions of this Design Guide, will comply with the requirements of the Building Code of Australia. These span tables have been prepared in accordance with standard engineering principles, the relevant test reports and Australian standards, i.e. -

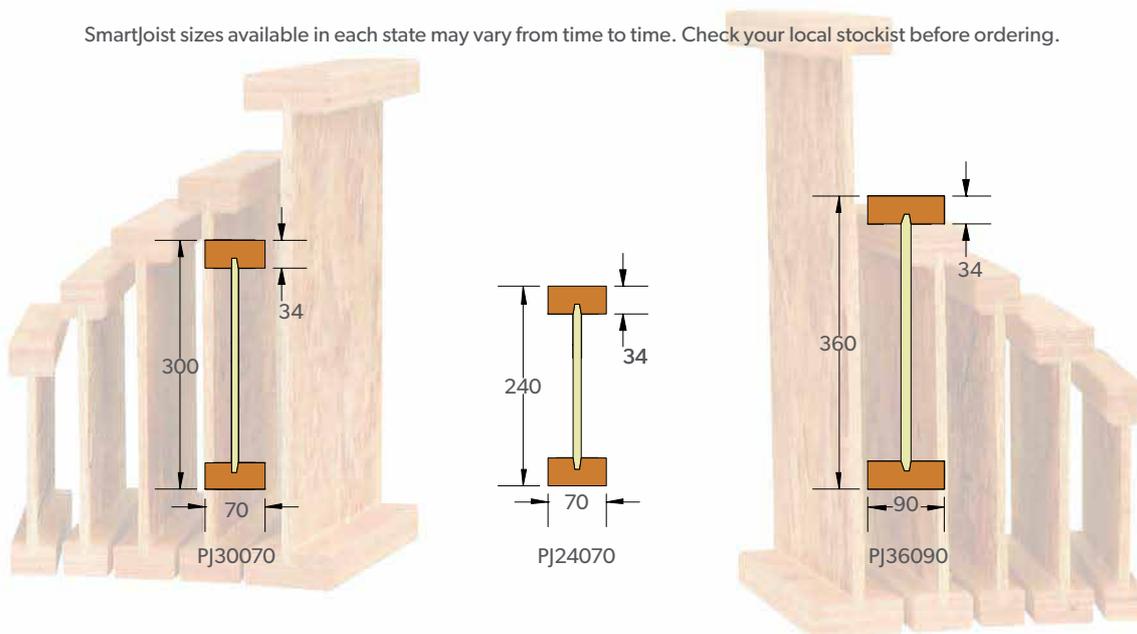
- AS 1684.1 Residential timber-framed construction
- AS 1170.1 Structural Design Actions – Permanent Imposed and other actions
- AS 1720.1 Timber Structures - Design Methods
- AS 4055 Wind loads for Houses
- ASTM D 5055 Standard specification for establishing and monitoring structural capacities of prefabricated wood I-joists.

Craig Kay.

Craig Kay PEng, EC1961, RPEQ5100, BPB0730, CC5635C, NPER
National Product Manager – EWP

Web thickness 70 mm flanges: 9.5 mm, 90 mm flanges: 11.5 mm

SmartJoist sizes available in each state may vary from time to time. Check your local stockist before ordering.



SmartJoist dimension tolerances: depth: +0 -3 mm, Flange width: +/- 1 mm, flange thickness: no plus limitation -2 mm.

The Strength is in the Engineering

The Prolam Engineered Timber System is made up of:

World class engineered timber products:

- i. Prolam
- ii. Prospan
- iii. Prolam I-Joist
- iv. Unique Structural Design, Detailing & Online Software

Prolam I-Joists

The strength is in the engineering

Strong. Stiff. Reliable. Prolam I-Joists are engineered for heavy performance. We start with ultrasonically graded LVL, bonded with exterior adhesive for more load carrying capacity.

The webs are made from stable, strong Oriented Strand Board (OSB) for superior strength and consistent performance. Prolam I-Joists are more uniform than solid sawn joists. They stay straighter and are manufactured with no camber, so there is no chance of crown down or upside down installation. They resist shrinking, twisting, warping and splitting for squeak resistant floors and quality roofs and ceilings.

Holes may be easily cut in the web according to the tables on page 22, allowing ducts and utilities to be run through the joists. Pre-punched 40 mm knockout holes are provided in the web for small diameter services or wiring.

Save time and money

Because they weigh less than solid sawn joists, Prolam I-Joists are easier to install, saving construction time and cost. Their greater load carrying capacity allows you to space them further apart, so it takes fewer to build the average floor or roof. And with five (4) depths from 200 to 360 mm, you will never have to compromise your design. So whether your plans call for cantilever beams in balconies, cathedral roofs or high pitched roof slopes, Prolam I-Joists are the perfect choice.

An environmentally sound choice

In addition to being cost effective, Prolam I-Joists are also an environmentally sound choice because they are made of a renewable resource – wood. So they are a better choice for building.

Prolam Online Software

Our unique Prolam Online, detailing and estimating software offers you unparalleled design and estimating capabilities with engineered timber. You will get accurate designs for a wide variety of applications, printouts and joist layouts.

Limitations of use – Prolam I-Joists

Prolam I-Joists are to be used in dry interior environments only, fully enclosed from exposure to exterior moisture. Prolam I-Joists are suitable for subfloor applications provided that the subfloor space is ventilated as per the BCA requirements. This means that Prolam I-Joists must not be exposed to environments where the equilibrium moisture content of the joist will exceed 18%. Prowood will not guarantee Prolam I-Joists that have been left exposed to the weather either prior to or during construction for more than 90 days.

Detailing such as cladding or lining must be used in moisture laden environments (commercial kitchens, bathrooms, wet industrial areas, saunas, swimming pool and spa rooms etc.) and constructed in such a way as to prevent exposure of the Prolam I-Joist to moisture.

Prolam I-Joists may be used in applications which are often exposed externally (gable ends, eaves, floor joists applications in elevated houses, cantilevered joists etc.) but must be sufficiently enclosed with a suitable cladding, lining etc. to completely prevent the exposure of the Prolam I-Joist to moisture.



Prolam®
I-Joists



**WE'RE PROUDLY
NEW ZEALAND
OWNED & OPERATED**

Product Warranty

Prowood guarantees that Prolam I-Joist products have been manufactured to exacting standards and are free from defects in workmanship and materials.

At Prowood, we take great pride in Prolam I-Joist products, so if you bring to our attention problems such as squeaks that you believe are caused by our products, we guarantee that a technical representative will contact you promptly to evaluate the issues and provide advice to help solve the problem.

Providing that any Prolam product is correctly designed, handled and installed, any problem caused by an unlikely defect will promptly be remedied at no cost to you.

General information about Floor Performance

The “feeling” that is identified when a person walks on a floor is very subjective. Some people want to feel a very stiff floor and others want some “give” so that it softens the footing. When people say the floor “bounces”, it may be vibrating. This sensation is often caused by lack of dead load such as furniture, direct applied ceilings or other materials to absorb or dampen the vibration.

The allowable spans shown in the tables of this manual have been designed to meet the strength and serviceability criteria in AS1684.1. This standard introduced a further serviceability equation into the design of floor joists which checked the deflection caused by a 1.0 kN load applied at mid-span. If the differential deflection of the joist relative to an adjacent joist exceeds 2.0 mm then the span is deemed to be such that the floor performance may be considered too bouncy for service.

Factors that can affect floor dynamic performance

- The choice of flooring system
- The depth, stiffness and mass of the joists
- Spacing of joists
- Fixing of sheathing to joists
- Stiffness and mass of floor sheathing
- Mass and stiffness of ceiling materials
- Method of installation
- Location and type of internal partitions and furniture.

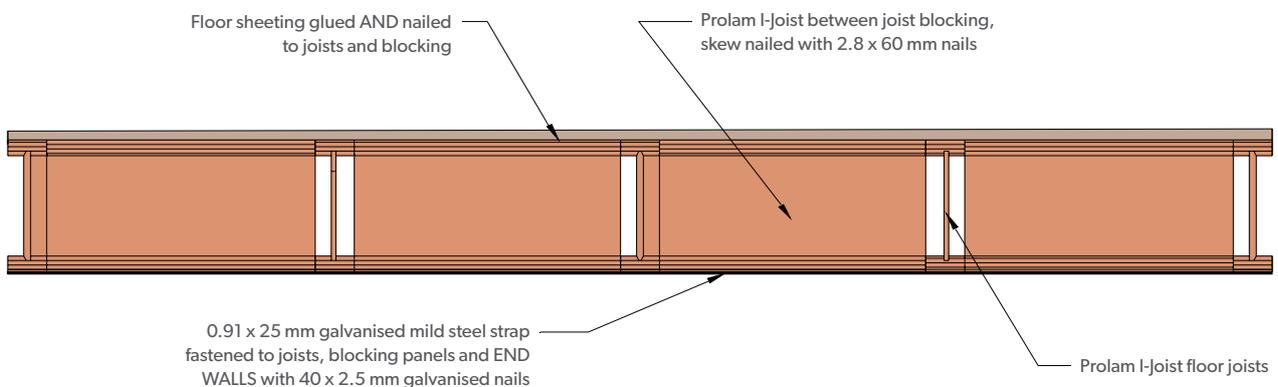
Factors that can improve floor dynamic performance

- Glue-nailed floors will perform better than floors secured by nails alone
- Deflection of the sheathing material between joists can be reduced by decreasing the joist spacing or using a thicker and/or stiffer sheathing
- Proper installation is essential for dependable performance. Adequate and level support for the joists is necessary, as is correct fastening of the joists and sheathing
- The installation of a ceiling to the bottom flange of the joists
- Between joist blocking can provide some improvement to floor dynamic performance. It is emphasised that for between joist blocking to be effective, it is important that the blocking is continuous, this being easily achieved by the addition of a continuous bottom strap such as hoop iron strapping which is also attached to the end walls.

If floor dynamic performance is a concern to either the client, designer or contractor, then the above variables can be altered to improve dynamic performance. Some stiff floors with very little dead load may tend to vibrate. This can generally be dampened by directly attaching the ceiling below the underside of the joists. Where there is no lining to the underside of the joists, it is recommended that between joist blocking be utilised to dampen this lightweight floor.

If between joist blocking is to be used to improve floor dynamic performance, it is recommended that a blocking system (at least midspan, 1/3 points for large open rooms) similar to the one shown below should be adopted.

Between Joist Blocking for Prolam I-joists



Prolam I-Joist Design/Effective Span

Normal structural analysis uses the centreline representation of the member. The term “span” can be defined in a number of ways and these are defined as follows:

Clear Span: This is the distance between the faces of any support. It is generally the one easiest to measure and read from the drawings.

Nominal span/centre-line span: This is the distance between the centre of the supports. This span is used to determine bending moments and deflections for continuous spanning Prolam I-joist members.

Design span/Effective span: This is the span used for single span members to determine the bending moment, the slenderness of bending members and the deflections. In AS 1720.1, this is the dimension referred to as “L”, and is defined below.

Design span/Effective span is the distance between:

- The centre of the bearing at each end of a beam where the bearing lengths have NOT been conservatively sized
- The centre of notional bearing that have been sized appropriately, where the size of the bearing IS conservative.

Diagram (a) shows beam where bearings have been designed appropriately. The effective span is taken as the distance between the centre of each bearing area.

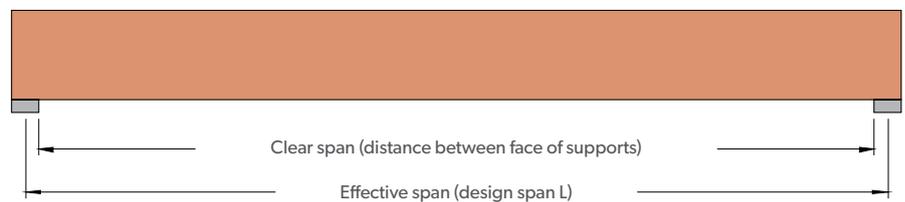
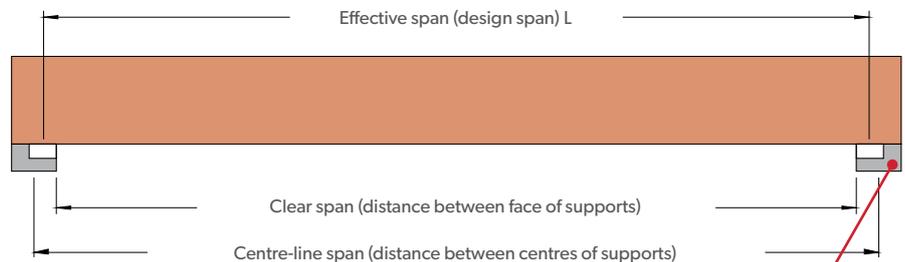
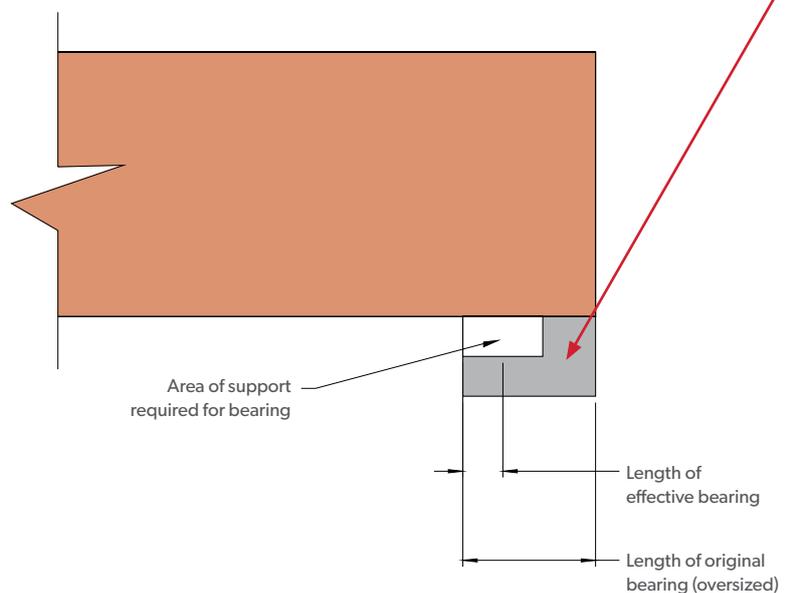


Diagram (b) shows beam where bearings at each end have been oversized. (This is frequently the case for beams that bear onto brickwork or concrete walls where the thickness of the wall is in excess of the area required to give the beam bearing capacity).



1. Calculate the minimum bearing required to carry the loads satisfactorily
2. Add minimum bearing length to “clear span” distance.



Recommended maximum spans for residential floors – General Domestic 1.5 kPa

Loadings: Permanent Loading G: self weight + 40 kg/m² + 0.6 kPa of live load permanently applied, live load Q: 1.5 kPa or 1.8 kN point live load

Joist Spacing (mm)		300	400	450	600	300	400	450	600
Prolam I-Joist Code	Self Weight (kg/m)	Maximum Floor Joist Span (m)							
		Single Span				Continuous Span			
PJ24070	4.0	5800	5400	5200	4850	6700	6200	6000	5500
PJ30070	4.3	6600	6100	6000	5500	7600	7100	6800	6300
PJ36090	5.9	7700	7200	7000	6500	9050	8400	8100	7500

In compiling the span tables in this manual, the requirements of the relevant New Zealand standards and codes along with established Industry standard design guidelines for Residential Construction have been followed. In particular, the following codes and references have been used:

- AS 1684.1 Residential timber-framed construction
- AS 1170.1 Structural design actions – permanent imposed and other actions
- AS 1720.1 Timber Structures - design methods
- AS 4055 Wind loads for houses
- AS/NZS 4063 Characterisation of structural timber
- ASTM D 5055 Standard specification for establishing and monitoring structural capacities of prefabricated wood I-joists.

Serviceability Criteria

Maximum dead load deflection:

Lesser of span / 300 or 15 mm ($j_2 = 2$)

Maximum live load deflection:

Lesser of span / 360 or 9 mm

Floor Dynamic Performance Criteria

Minimum natural frequency: 8 Hertz

Maximum differential deflection between joists of 2 mm under a concentrated load of 1.0 kN mid-span.

Flooring

Spans are suitable for solid timber, particle board and ply flooring. Floor sheathing glued and nailed to the joists will improve floor rigidity. Where a heavy overlay material is to be applied, such as thick mortar bed tiled or slate floors, the permanent load allowance should be increased to 1.2 kPa. A reduction of joist spacing can be used to accommodate this extra permanent load. A satisfactory result can be achieved by adopting the maximum spans for 600 mm and 450 mm spacings but installing the joists at 450 mm and 300 mm spacings respectively.

Continuous Spans

For beams which are continuous over two unequal spans, the design span and the “resultant span description” depend on the percentage difference between the two spans as shown below:

Span difference	Effective span	Resultant span description
10% max	main span	continuous
10 – 30%	1.1 x main span	continuous
above 30% diff	main span	single

$$\text{SPAN DIFFERENCE} = \frac{(\text{main span} - 2\text{nd span})}{(\text{main span} + 2\text{nd span})} \times 100$$



Nailing

Use only the listed galvanised bracket nails. All holes are to be filled with the specified nails in order to achieve the stated hanger capacity. Alternatively, screw with 35 x 6 gauge bugle-head or wafer-head wood screws. The joist hangers below have been developed specifically for Prolam I-Joists. The joist hangers and nails are available from Prowood as part of an order. It is not recommended that joist hangers other than those listed below be used with Prolam I-Joist.

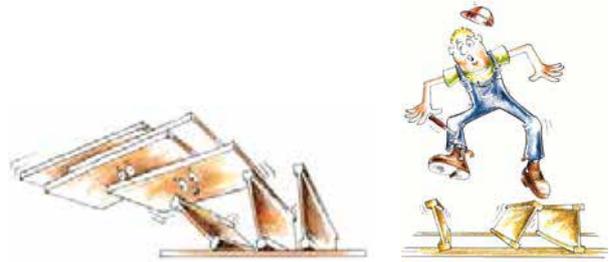
Prolam I-Joists brackets in areas shaded require web stiffeners as per detail F13

Prolam I-Joist	Face Mount Code	Hanger Capacity økN*	Face Nail Holes	Nail Size	Top Mount Code	Hanger Capacity økN*	Face Nail Holes to Support	Top Nail Holes	Nails to Joist	Nail Size
	Single Joist Face Mounts				Single Joist Top Mount					
PJ24070	24070F	7.8	10	3.75 x 40	24070T	4.8	2	4	2	3.75 x 40
PJ30070	30070F	9.3	12	3.75 x 40	30070T	4.8	2	4	2	3.75 x 40
PJ36090	36090F	10.9	14	3.75 x 40	36090T	4.8	2	4	2	3.75 x 40
Double Joist Face Mounts				Double Joist Top Mounts						
2/PJ24070	24070DF	7.8	10	3.75 x 40	24070DT	4.8	2	2	4	3.75 x 40
2/PJ30070	30070DF	8.7	12	3.75 x 40	30070DT	4.8	2	2	4	3.75 x 40
2/PJ36090	N/A				36090DT	5.7	2	4	2	3.75 x 40
Skewed Left or Right (Face Mount)										
Prolam I-Joist	SmartFrame Code	Hanger Capacity økN*	Face Nail Holes	Nails to Joist	Nail Size					
PJ24070	N/A									
PJ36090	36090FR to FL	7.8	10	2	3.75 x 40					
ALL	LVSIA	5.5	4	1	12g x 35 screw					

Notes:
 * Hanger capacity is based upon dead load + floor live load for a supporting beam of joint strength JD5.
 $k1 = 0.69$, Capacity factor $\phi = 0.85$.
 For permanent loads, the above value should be multiplied by $0.57/0.69 = 0.82$.

Safety Warning

Do not allow workers or loads on Prolam I-Joists until all blocking, hangers, rim joists, nailing and temporary bracing are installed as specified below. Serious accidents or injury can result from failure to follow these guidelines.

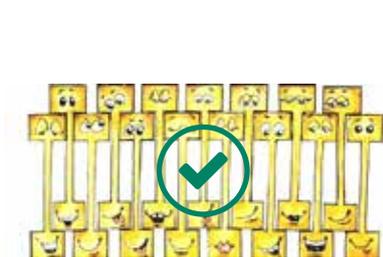


Accidents can be avoided under normal conditions by following these guidelines:

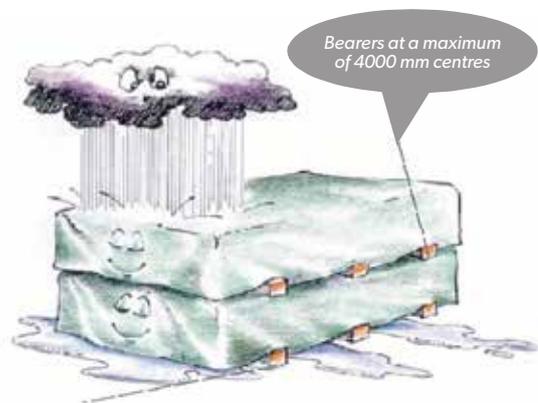
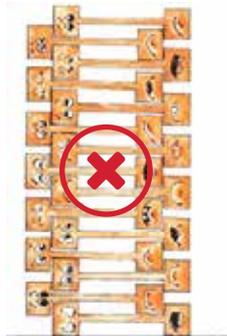
1. Brace each joist as it is erected. Joists must be nailed to supports and all hangers, blocking, rim joists. X - bridging at supports must be completely installed and properly nailed. (see general notes and details).
2. Brace the ends of cantilevers (overhangs) with closure panels, rim joist or x - bridging (see general notes and details).
3. Lateral brace the top flange of each joist, to prevent sideways buckling or rollover which may occur under light construction loads, such as a worker and/or a layer of un-nailed sheathing. Fully installed permanent sheathing or temporary struts to the top flange of each joist (see 'Typical Smartjoist floor framing') can accomplish lateral bracing. Temporary struts must be nailed to a lateral restraint at the end of bay such as a braced wall or temporary (or permanent) sheathing nailed to the first 1200 mm of the joist at the end of the bay (see 'Typical floor or roof framing').
4. Permanent sheathing must be completely installed and properly nailed before additional loads can be placed on the system.
5. **The integrity and safe use of these products can be seriously impaired if they are damaged. Do not install any damaged products. Contact your Prowood representative on 0508 776 526 if any product damage is noted.**

Handling & Storage of Prolam I-Joists

- Store Prolam I-Joists flat on a hard, dry surface
- If surface isn't paved, the ground should be covered with a polythene film
- Keep covered with waterproof material that allows bundles to "breathe"
- Use bearers (bolsters) between the ground and the first bundle (4 metre max spacing)
- Use 100 x 50 timber flat between bundles at same spacing as bolsters
- Take great care to rewrap remaining material after opening bundles
- Wood "grows" in thickness and depth when allowed to get wet....KEEP DRY!
- Wood with high MC has short term reduction in Characteristic Strengths KEEP DRY!
- Under NO circumstances are stored Prolam I-Joists to be in contact with the ground.



Prolam I-Joists should be stacked in the upright position to avoid any damage



Use bearers to keep stacked material away from damp surfaces. Align bearer vertically.

Durability and Exposure to Moisture

Prolam I-Joists are manufactured with Douglas Fir (Oregon) flanges with OSB webs, both having a durability rating of class 4, which is the same rating as some Ash type Eucalypts.

Moisture effects on Prolam I-Joists

Prolam I-Joist is supplied WITHOUT any short term construction sealer, but once framed into a structure may be exposed to the weather for a limited time (not greater than 3 months) without negative affect, BUT, it may exhibit some effects of this exposure.

The wood fibre in Prolam I-Joists, like all wood products, is hygroscopic, which means it has an affinity for water. The wood fibre in Prolam I-Joist will readily take up and release moisture in response to changes in the local environment. Moisture exposure will lead to dimensional change. While the products will withstand normal exposure, excessive exposure during distribution, storage or construction may lead to dimensional changes that affect serviceability. These changes include twisting, bowing or expansion to dimensions to beyond the specified tolerance of the product in the "as-manufactured" condition.

As an organic material, mold and mildew may grow on untreated wood products if moisture is present. Prolonged periods of high moisture may also support the growth of wood decay fungi, which is another reason to follow proper methods of storage and handling of Prolam I-Joists. The table below shows the moisture content of Prolam I-Joists as a function of humidity.

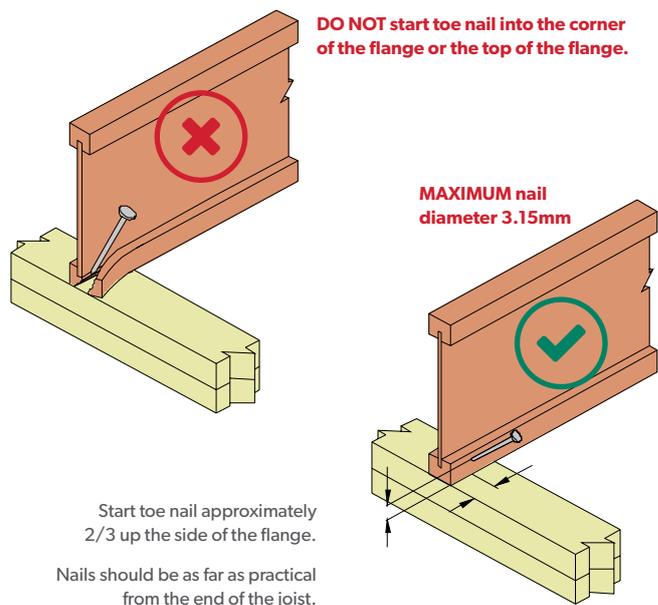
Wetting during construction may lead to temporary elevated moisture content and dimensional changes. Once covered, the Prolam I-Joist will ultimately dry and re-equilibrate to the ambient humidity conditions, but some expansion or swelling may remain after drying.

Moisture content of wood products % ¹		
Relative Humidity%	LVL Flange MC	OSB web
10	1.2	0.8
20	2.8	1.0
30	4.6	2.0
40	5.8	3.6
50	7.0	5.2
60	8.4	6.3
70	11.1	8.9
80	15.3	13.1
90	19.4	17.2

¹ Approximate moisture content at 21°

Prolam I-Joists General Notes

1. Except where otherwise noted, 30 mm minimum bearing is required at joist ends and 42 mm minimum bearing is required at intermediate supports.
2. Nail joists at each bearing with 2 of 3.15ø x 65 nails, using one each side placed 30 mm from the end to avoid splitting.
3. Prolam I-Joist blocking or Prorim - face nail to bearing plate with 3.15ø x 65 nails at 150 mm centres. Nail rim joist to the end of the top and bottom flange of each Prolam I-Joist with 1 3.15ø x 65 nail, use 1 3.75 x 75 nail top and bottom with joists with 58, 70 or 90mm wide flanges.
4. 4.25 mm Prorim - toe nail to bearing plate with 3.15ø x 65 nails at 150 centres or 4.5ø x 75 nails at 300 centres. Nail rim to the end of the top and bottom flanges of each Prolam I-Joist with 1 3.15ø x 65 nails.
5. Sheathing nailing to top flange (Joists must be fully braced before sheathing is nailed)
 - Space 2.8ø x 65 and 3.15ø x 65 nails no closer than 50 mm per row.
 - Space 3.75 x 75 nails no closer than 75 mm.
 - Maximum nail spacing: 300 mm



Prolam I-Joists General Notes continued

6. Backer blocks at hanger details:
 - 58 mm flange - 2 pieces of 12 mm ply
 - 70 mm flange - 2 pieces of 15 mm ply
 - 90 mm flange - 2 pieces of 19 mm ply
7. See double Prolam I-Joist detail F15 for filler blocks. Nail Joists together with two rows of 3.75 ϕ x 75 nails on each side of double joist at 300 mm centres (Clinch if possible). A total of 4 nails per 300 mm is required. If nails can be clinched, only 2 nails per 300 mm is required.
8. All joists require lateral support at end bearings using blocking or rim material.
9. The top flanges must be kept straight within 10 mm of the true alignment.
10. See web stiffener detail F13 for web stiffener attachment at supports. Web stiffener requirements for concentrated loads in excess of 4.5 kN, applied at the top flange of the joist, requires additional consideration.
11. When required, install web stiffeners to joist (see detail F13) prior to placing joist in the hanger, then nail hanger to joist.
12. All roof details are valid to a maximum angle of 35° (as per AS1684).
13. All nails are steel nails complying with AS 2334 - 1980 Steel nails - Metric series. Nail gun nails of similar length and diameter may be substituted for the above provided that they are manufactured with properties equivalent to the nails in the above code.
14. Install all hangers to the manufacturers installation instructions, taking particular attention to the use of the correct nails. Never use clouts or brads.
15. Prescriptive code requirements for mid span blocking of solid timber joists are not applicable to Prolam I-Joists.

Typical Prolam I-Joist Floor Details

Blocking and Lateral Restraint General Notes

Prolam I-Joists designed and constructed as per this Design Guide do not require mid-span blocking. The exception to this is for lightweight subfloors where there is no lining to the underside of the joists. For more information on this topic, see page 3 'About Floor Performance'.

Blocking within a structure falls within two (2) quite distinct stages: **Temporary** or during construction blocking to prevent roll over of joists before the installation of floor sheeting.

Permanent blocking to provide resistance to racking loads through the floor diaphragm, transfer of vertical wall loads and to provide torsional resistance to the end of the joist.

The provision contained within AS1684 Residential timberframed construction code dealing with blocking for deep joists, is "during construction" or "temporary" blocking, designed only to prevent the roll over of the deep joists prior to the floor sheeting being attached. This level of blocking can form a part of any overall blocking system, but was never intended to provide the total amount of racking resistance or vertical load transfer requirements within this floor diaphragm.

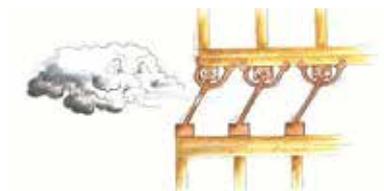
The lateral bracing requirements of the structure, unless there is full blocking of exterior walls, must be calculated in each individual case. Advice on this matter is obtainable from AS1684 Residential timber-framed construction code.

1.0 Joists bearing onto external walls

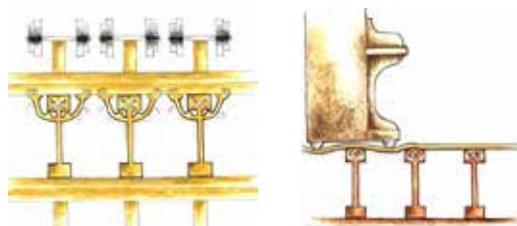
1.1 Loads at Joist/support connection

The ends of floor joists that bear onto a support experience external loads other than the floor dead and live loads, as shown. Any I-Joist, with its small cross sectional area, needs to have its end bearing capacity considered as part of the design process.

Further, as a holistic approach to the consideration of the lateral stability of the complete structure, it is necessary to consider the availability of racking and shear resistance through the floor diaphragm.



1. Racking and shear effects due to wind and earthquake loads
2. Vertical loads on joists due to upper wall, floors and roof.
3. Unsightly deflections in the edges of unsupported sheet flooring may be experienced if heavy items of furniture are placed close to sheet edges.



Typical Prolam I-Joist Floor Details continued

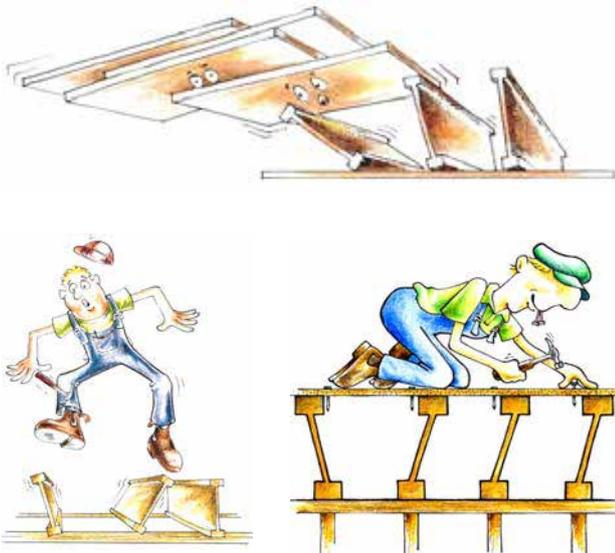
1.2 Stages of Blocking/Bracing

1.2.1 Temporary (during construction) end blocking

Temporary or during construction blocking of the ends of joists over external wall must comply with the requirements as shown in the "Safety Warning" on page 7 and as shown in the "Typical Prolam I-Joist floor framing" diagram on page 12.

This is summarised as:

- Temporary struts, fastened to top of Prolam I-joist, connected back to braced supports.
- Temporary floor sheeting nailed to the first 1200 mm of joists at the end of the bay, in combination with struts, if no connection to a braced wall can be made.

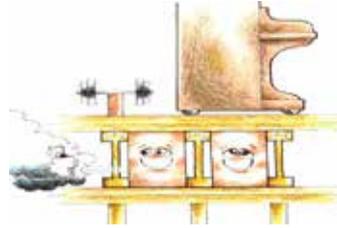


1.2.2 Permanent End Blocking/Bracing

Permanent blocking (bracing) to be effective in providing adequate transfer of racking and shear loads through the floor diaphragm must comply with the details as shown in "Typical Prolam I-Joist framing" diagram on page 12. In essence, fully block the ends of all joists at their bearing point on external walls, as per one of the options shown in details F1- F4.

This permanent blocking/bracing provides:

1. A satisfactory mechanism to transfer racking loads through the floor diaphragm.
2. Vertical load transfer independent of the floor joist.
3. Support to the end of the floor sheeting (Platform floors only). Heavily loaded furniture legs have been known to cause large deflections and even failures at the edges of sheet flooring.
4. Torsional restraint to the end of floor joists, improving the joists structural performance.



2.0 Interior Supports

2.1 Ends of simple spans

Where Prolam I-joists are discontinuous over interior supports, install the temporary strut bracing as per "Safety Warning" on page 7.

2.2 Continuous spans

Continuous joists over internal supports do not require blocking, other than the temporary top flange struts as shown in the "Safety Warning" on page 7, except in the following circumstances:

- Load bearing walls bear onto the joists at their support. (Details F7 or F8 apply)
- Shear resistance is required in internal walls (This is a function of shear resistance, and is not related to the structural adequacy of the joist itself).

3.0 Blocking and Wall Plates

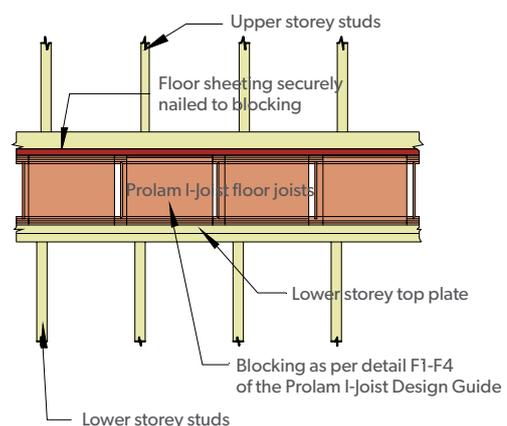
Wall plates in the frame are required to transfer vertical loads into the support structure below. These wall plates may be supported at 450 or 600 mm ctrs, thus acting as a beam between supports, bending about its weaker axis.

When concentrated loads act at the centre of this wall plate, the bending and deflection effects can be quite significant.

The full blocking of external and load bearing walls, as shown in details F1-F4, can act as a beam transferring these loads to the support structure below, thus reducing the beam effect of the wall plates.

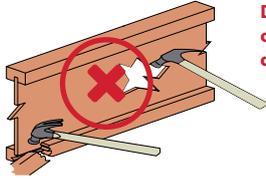
Unless there is a requirement for double wall plates for a reason **other** than the beam effect between supports, walls blocked as per detail F1-F4 and general notes #2, #3, and #4 provide sufficient.

Wall and Roof Loadings

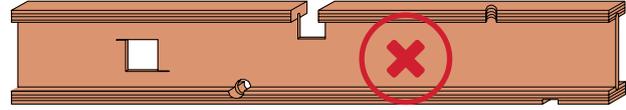


Field Repairs to Damaged Prolam I-Joists

Don't make holes with hammer other than pre-punched knockouts



Don't hammer on flanges and damage joint



**Do not cut or notch flanges
Do not over-cut holes in web**

Note: Prolam I-Joists are sophisticated Engineered Timber products, and must be treated accordingly. Damage to key components, while affecting only a small percentage of the cross section may be sufficient to render the Prolam I-joist unsuitable for the purpose.

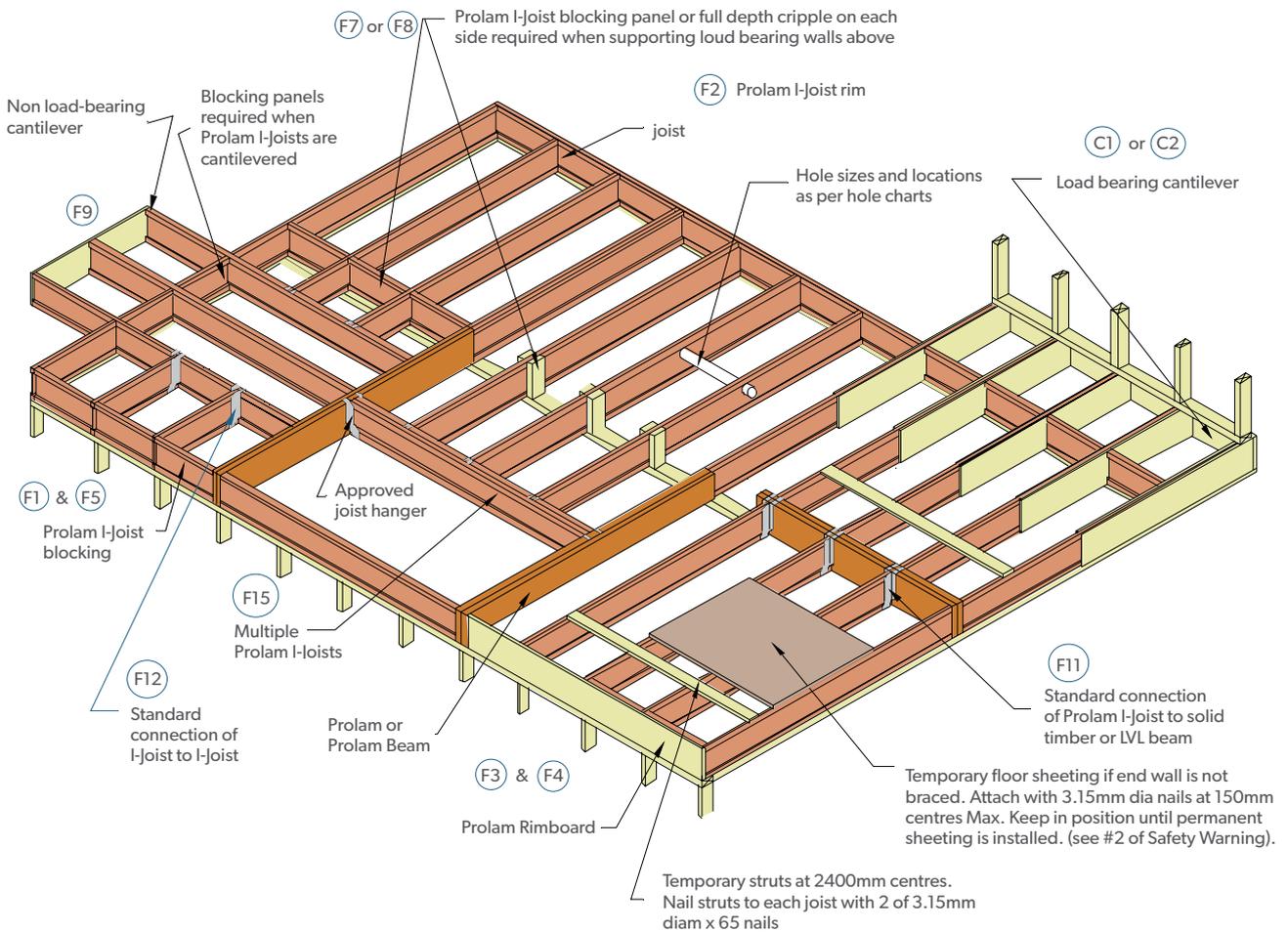
Flange Damage

- Flange damage becomes more critical the nearer it is to mid-span or an interior support. Flange damage is less critical in close proximity to an end support.
- How much flange damage is acceptable? A rule of thumb is "If you have to ask, it's too much". A saw kerf that knicks the corner of a flange on one lightly-loaded joist could well be acceptable.
- A joist with unacceptable flange damage cannot be repaired, rather a new joist must be added to take its place. The damaged joist does not have to be removed. Consult Prolam I-joist tables to find an acceptable new joist that is shallower than the damaged joist so installation is easier. Consider double and triple joists. If the damaged joist is multi-span, the new joist only needs to go across the span(s) where the damage occurs.
- A single damaged joist can sometimes be trimmed off of adjacent undamaged joists.

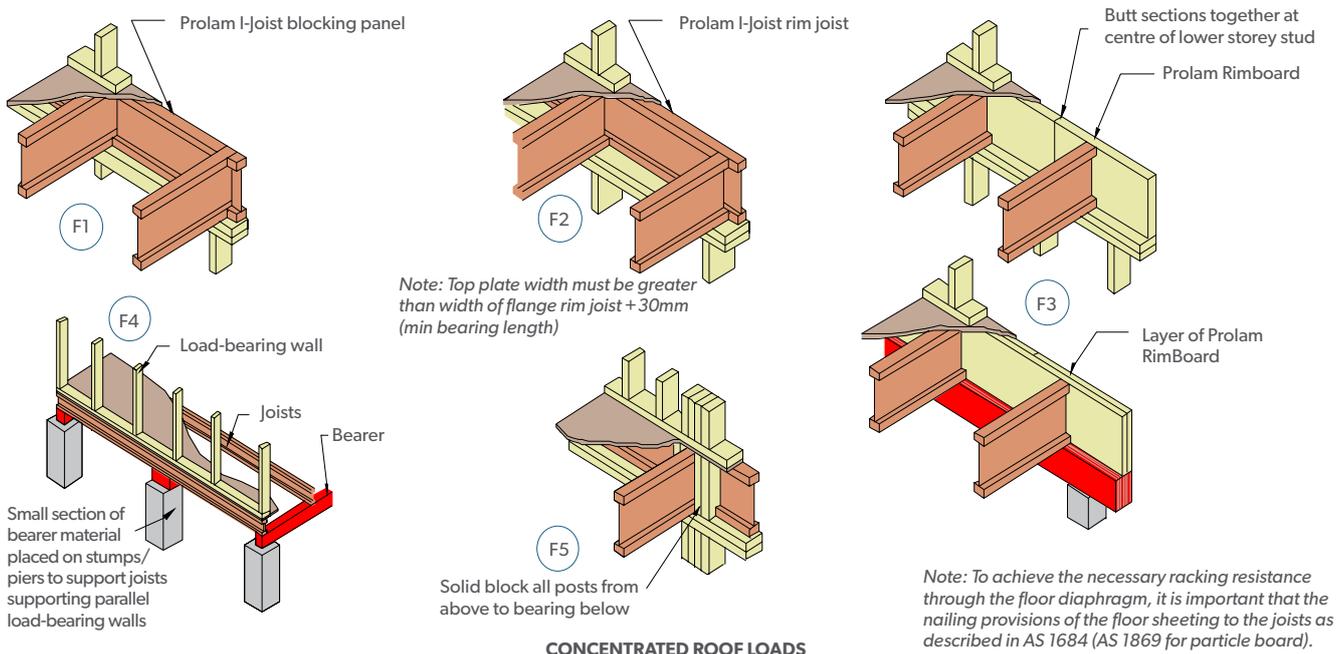
Web Damage

- Web damage becomes more critical the nearer a support. Web damage is less critical near mid-span.
- Web holes can be too big to repair. A flange-to-flange rectangular hole longer than 450 mm located at midspan probably warrants a new joist. A 150 mm round hole located right by a support probably warrants a new joist. Consult Prolam I-joist tables to find an acceptable new joist that is shallower than the damaged joist so installation is easier. Consider double and triple joists. If the damaged joist is multi-span, the new joist only needs to go across the span(s) where the damage occurs.
- A single damaged joist can sometimes be trimmed off of adjacent undamaged joists (run a calculation within the Prolam software).
- Damage that could be confidently repaired in a single, isolated joist, might be judged too severe to repair if several, adjacent joists are involved.
- If several small holes violate the 2x diameter proximity rule, but would fit inside a single acceptable hole, then the group of small holes is OK.
- Hole repairs generally require a reinforcement that covers the full depth of the web and extends at least 300 mm past each side of the hole.

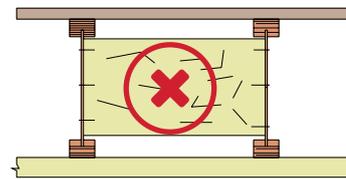
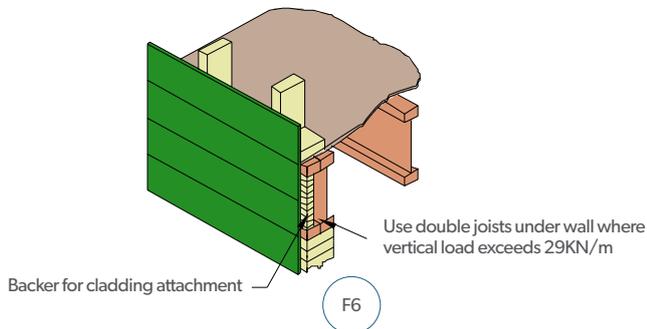
Typical Prolam I-Joist Floor Framing



Typical Prolam I-Joist Floor Construction Details



Typical SmartJoist floor construction details continued

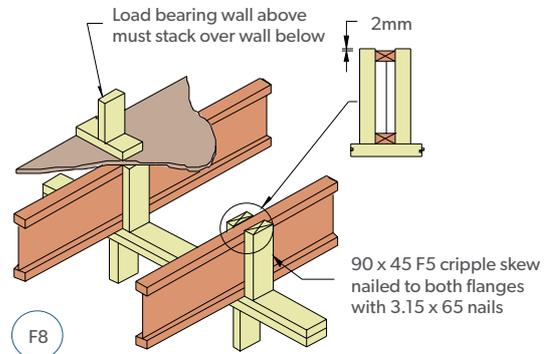
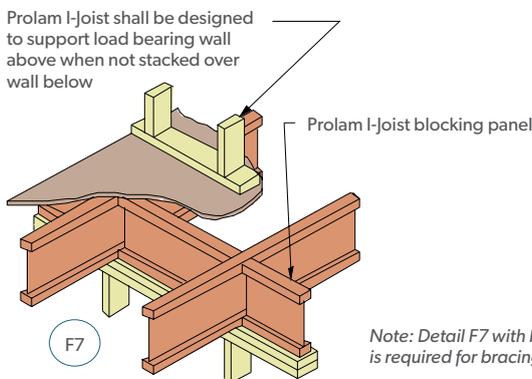


WARNING - Correct blocking for Prolam I-joists

GREEN TIMBER SHALL NOT BE USED UNDER ANY CIRCUMSTANCE

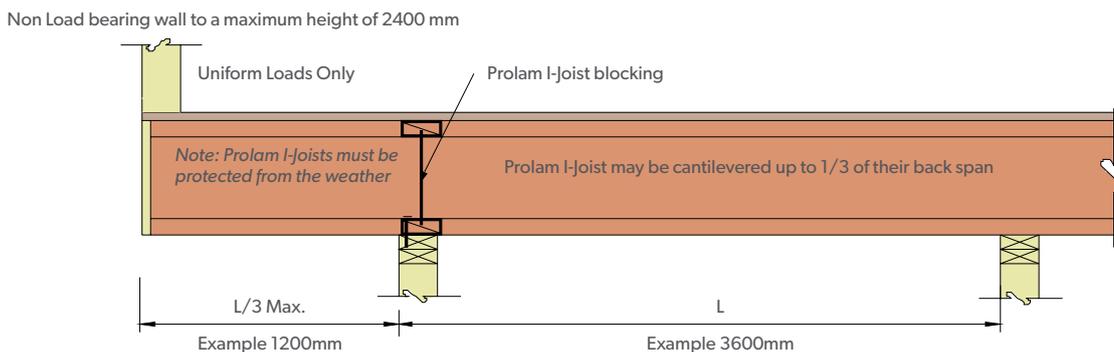
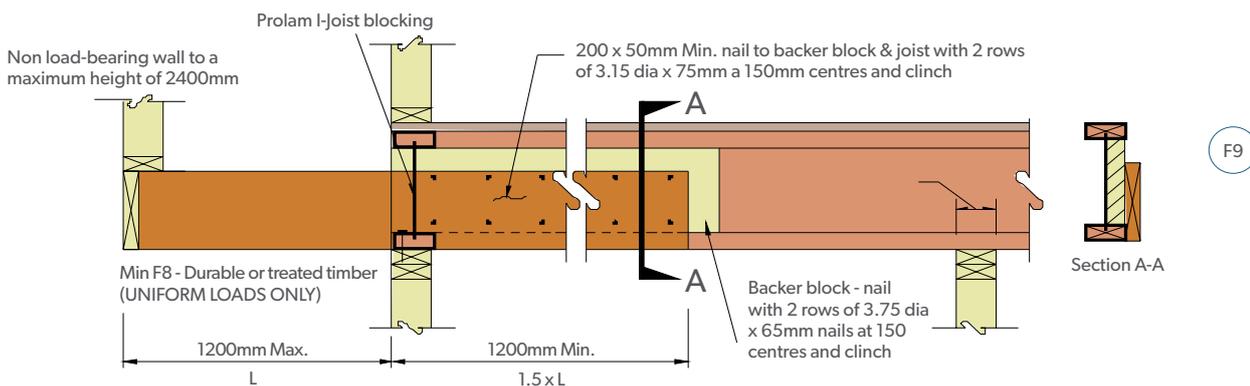
Note: All blocking shall be carried out as per details F1-F3, with blocking to extend to both flanges and skew nailed with 3.15ø x 65 nails, one each side of top and bottom flange

Interior Load Bearing and Bracing Walls



Non Load Bearing Cantilever Details (Balconies)

Example cantilever spans and minimum back spans for this details are shown in the table on the next page



FOR CANTILEVERS SUPPORTING LOAD BEARING WALLS, SEE DETAILS C1 OR C2

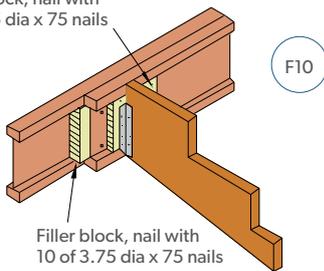
Cantilvered Balconies (as per detail F9)

Loadings: Permanent Loading G: self weight + 40 kg/m² + 0.6 kPa of live load permanently applied, live load Q: 2.0 kPa or 1.8 kN point live load, 1.5 kN/m acting at end of cantilever.

Balcony Cantilevers - Maximum cantilever and minimum back span (m)								
Joist Spacing (mm)	300		400		450		600	
	Cantilever	Back Span						
H3.2 Prospan LVL 15								
140 x 42	1.0	1.5	1.0	1.5	1.0	1.5	0.9	1.4
190 x 42	1.2	1.8	1.1	1.7	1.1	1.7	1.1	1.7
240 x 42	1.7	2.6	1.6	2.4	1.6	2.4	1.5	2.3
290 x 42	2.1	3.2	2.0	3.0	2.0	3.0	1.9	2.9

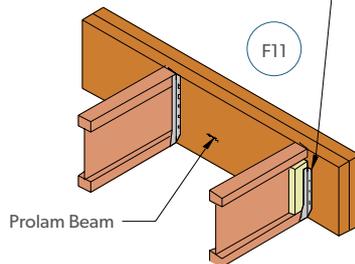
Backer and Filler Blocks

Backer block, nail with 10 of 3.75 dia x 75 nails



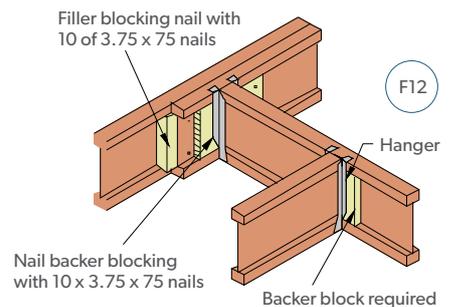
F10

If the sides of the hanger do not support the top flange, web stiffeners as per detail F13 are required



F11

Filler blocking nail with 10 of 3.75 x 75 nails



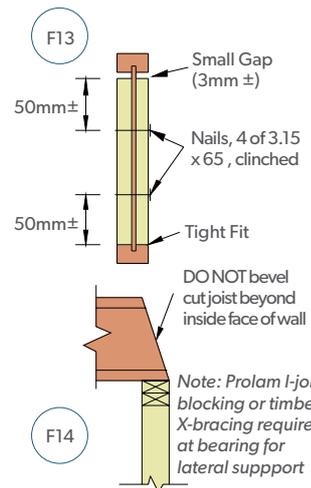
F12

Filler Blocks and Web Stiffeners			
Prolam I-Joist code	Recommended filler block	Web Stiffener Materials	
		Stiffener	Nails
PJ24070	150x58 LVL	2/15x60mm ply	4-3. 15x65
PJ30070	2/190x45	2/19x60mm ply	5-3. 15x65
PJ36090	2/240x45	2/19x60mm ply	5-3. 15x65

Web Stiffener

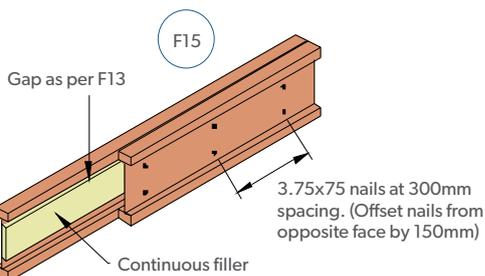
Notes:

- Use plywood sheathing for web stiffener with face grain parallel to long axis of the stiffener.
- Filler blocks noted are for the general requirements of the details within this design guide.
- Leave 3 mm gap between top of filler blocks and bottom of top flange.



F13

F14



F15

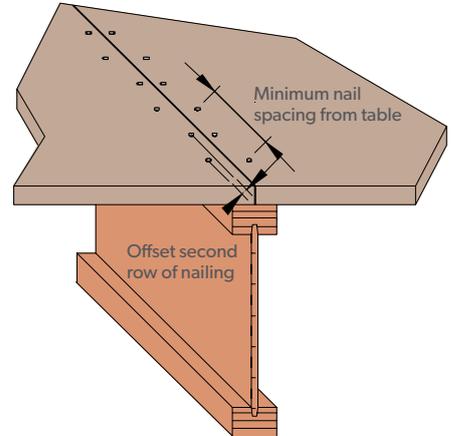
Double Prolam I-Joists

- Support back of web during nailing to prevent damage to web/flange connection
- Filler block is required full length of joist
- Nail Joists together with two rows of 3.75ø x 75 nails on each side of double joist at 300 mm centres (Cinch if possible). A total of 4 nails per 300 mm is required. If nails can be clinched, only 2 nails per 300 mm is required.

Fastener Spacing

Minimum single row nail spacing into Prolam I-joist flanges

Prolam I-joist flange width					
Nail size	40mm	44mm	51mm flange	58-70mm	90mm flange
2.8 x 65	70	65	50	50	50
3.15 x 65	100	90	75	75	75
3.15 x 75	100	90	75	75	75
3.75 x 75	130	115	100	100	100
4.5 x 100	NA ¹	NA ¹	NA ¹	NA ¹	100



Notes:

1. Nailing of bottom plate at 100 mm centres through floor sheathing and into top flange is permitted.
2. Minimum nail spacing is shown above, maximum nail spacing is 300mm centres.
3. Tighter effective nail spacing may be obtained by offsetting nail rows a minimum of 12 mm and maintaining a 10 mm minimum edge distance.

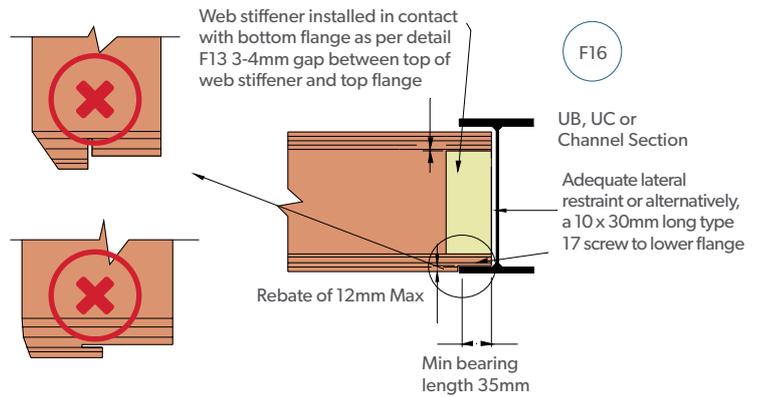
Limited End Notching at Supports

The cutting of notches in the ends of joists may reduce the allowable end reaction of the SmartJoists.

The amended end reaction capacities of Prolam I-joists with a 12 mm notch are as follows:

- Without web stiffeners - 80% of allowable end reaction
- With added web stiffeners (as per detail F13) - Full end reaction capacity.

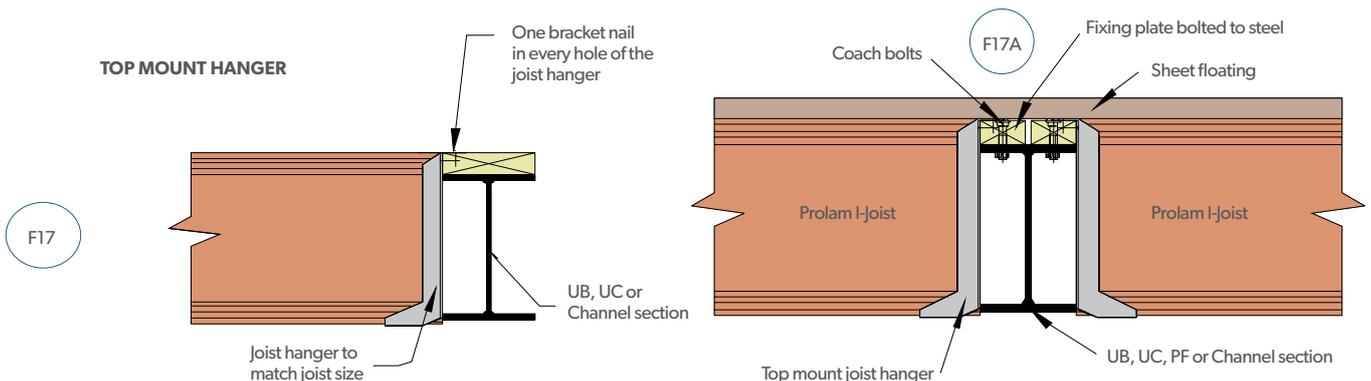
DO NOT OVER CUT FLANGES. SUBSTANTIAL REDUCTIONS IN CAPACITY MAY OCCUR IF FLANGES ARE OVER CUT.



To maintain the end reaction capacities above, end flange notching is strictly limited to:

1. Notch depths NOT greater than 12 mm
2. Notches cleanly cut - NO over cutting
3. Notch length not to exceed more than 5 mm past the support.

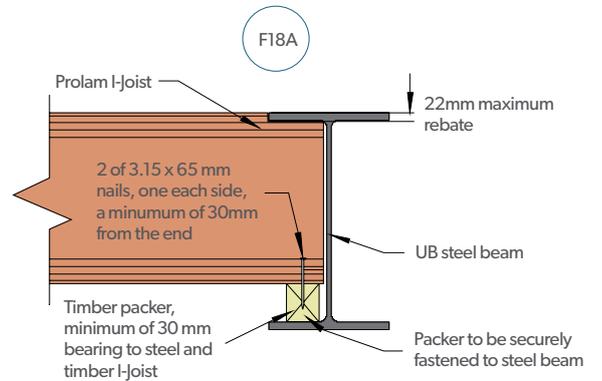
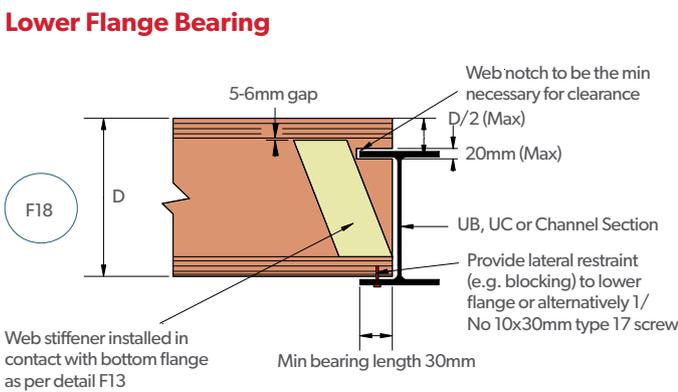
Example Fixing of Prolam I-joists to Steel Beams



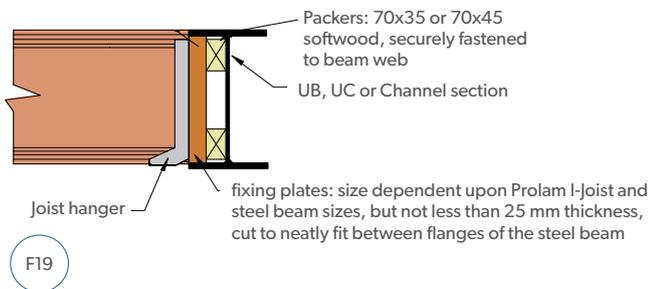
Example Fixing of Prolam I-Joists to Steel Beams

continued

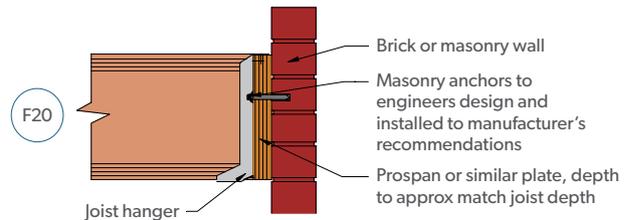
Lower Flange Bearing



Face Mount Hanger

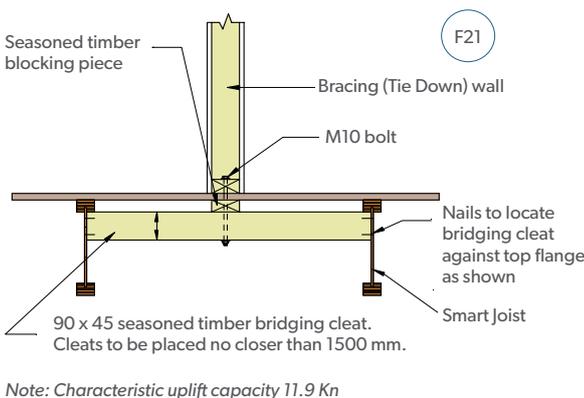


Example fixing of Prolam I-joists to brick or masonry walls

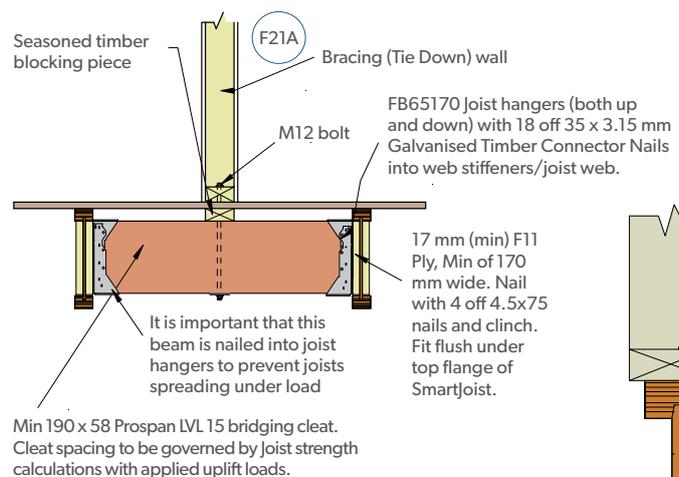


Tie Down (Bracing Wall) Details

The tie-down needs of the structure are related to the applied wind loads. Reference should be made to AS 1684 for further guidance on this issue. The general details relating to the tie-down provisions of solid end section timber may be adopted for SmartJoists, except that under NO circumstances is it permitted to bolt through either the top or bottom flange, except when the joist is fully supported upon a wall plate or similar as shown below.



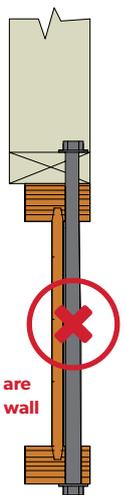
Note: Characteristic uplift capacity 11.9 Kn



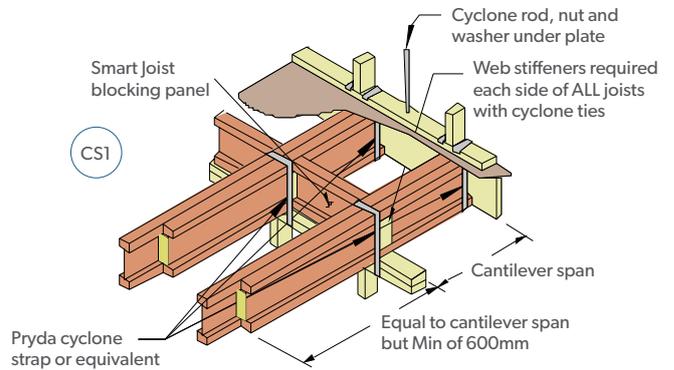
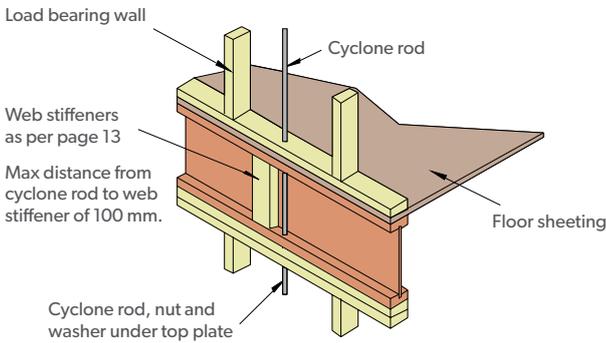
Min 190 x 58 Prospan LVL 15 bridging cleat. Cleat spacing to be governed by Joist strength calculations with applied uplift loads.

Note: Max force transfer of system 30.0 kN (It is essential that Prolam I-joist is analysed for these extreme loads)

DO NOT DRILL THROUGH EITHER FLANGE of Prolam I-joists unless they are fully supported on wall plate or similar

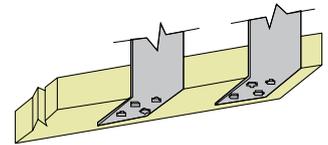


Cyclone Rod Tie Down for Cantilevered Prolam I-Joist Floors



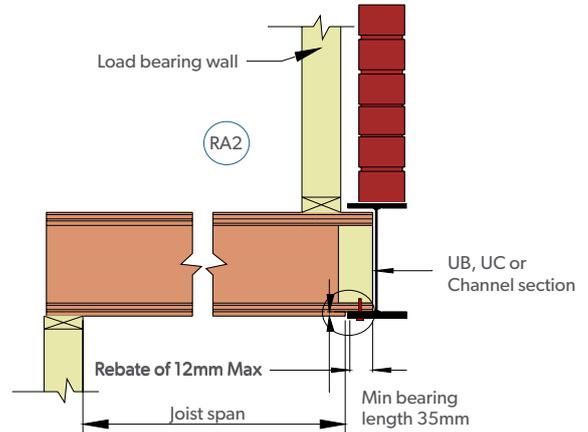
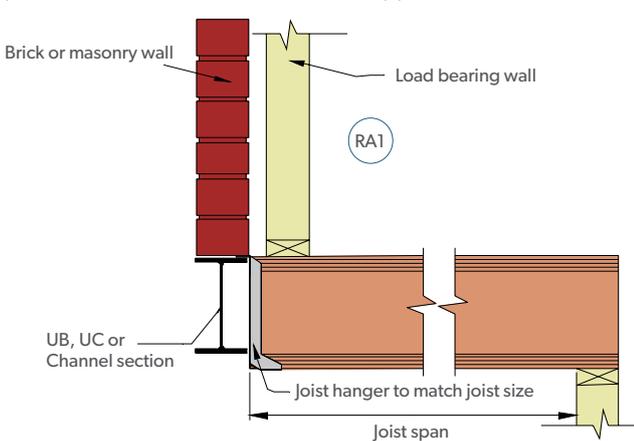
Cyclone Strap Capacities

Where the strap ends of the cyclone strap are wrapped around the wall plate or other timber member and are fixed with 4 of 3.15 \varnothing x 35 nails, the design capacity $\varnothing N_j$ of 15.3 kN is applicable, regardless of the timber joint group. Tests have proven that bending the legs of cyclone straps around the timber increases the ultimate load capacity. While double joists shown in the above diagram, it is only necessary when loads exceed the capacities of single joist cantilevers.



Joist/Beam Connections Supporting Offset Load Bearing Walls

Modern building designs frequently call for the upper storey of a two storey dwelling to be set back from the lower wall to allow sufficient light access to all areas of the building. Provided that the Prolam I-joists have been designed to support this offset load, no special provisions need to be made for their support EXCEPT in the following support conditions:



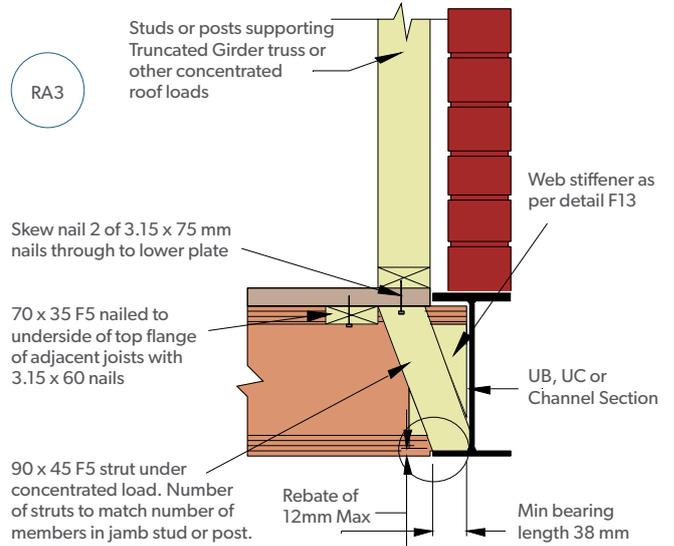
Maximum Roof Area Supported (m²) based upon worst case of 40mm flange width (conservative for wider flanged joists)

Joist supported on joist hanger RA1									Lower flange bearing RA2							
Joint spacing (mm)	300	400	450	600	300	400	450	600	300	400	450	600	300	400	450	600
Joint span (mm)	Sheet				Tile				Sheet				Tile			
3500	21.7	15.0	12.8	8.2	9.6	6.7	5.7	3.6	6.9	6.4	6.2	5.3	3.1	2.9	2.8	2.4
4000	21.1	14.5	12.3	6.9	9.4	6.4	5.5	3.1	6.7	6.2	6.0	4.6	3.0	2.8	2.7	2.0
4500	20.5	13.9	11.7	5.7	9.1	6.2	5.2	2.5	6.6	6.0	5.7	3.9	2.9	2.7	2.5	1.7
5000	20.0	13.4	10.4	4.4	8.9	5.9	4.6	2.0	6.4	5.8	5.1	3.1	2.9	2.6	2.3	1.4
5500	19.4	12.1	9.1	3.2	8.6	5.4	4.1	1.4	6.3	5.3	4.6	2.4	2.8	2.4	2.0	1.1

Support for Concentrated Loads Joist/Beam Connections Supporting Offset Load Bearing Walls

Concentrated loads from any source such as girder trusses MUST be transferred through the floor space WITHOUT adding extra vertical loads to the ends of the SmartJoist at its bearing support.

One example of transferring these loads is the use of inclined timber struts as shown in the detail opposite. Struts must be a tight fit and at a minimum angle of 60° to the horizontal.



Beams Supporting Prolam I-Joists Multiple Member Laminations

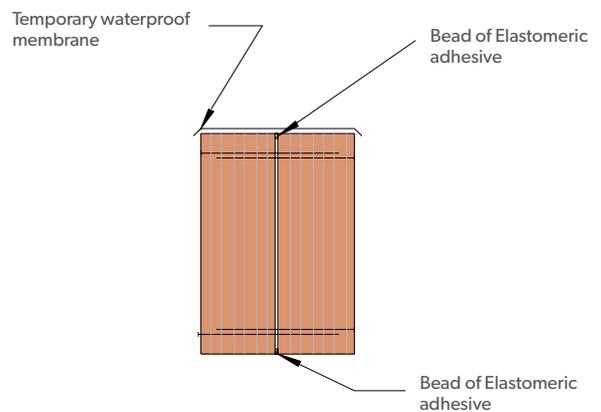
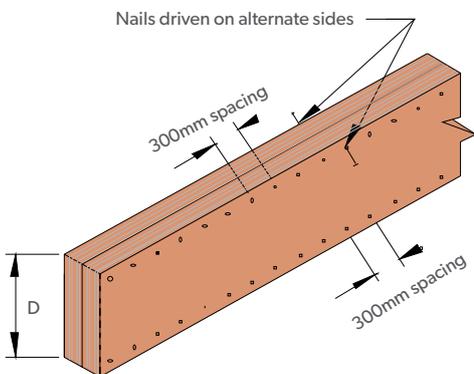
Vertical laminations may be achieved by adopting the procedures described in clause 2.3 of AS1684, however these procedures should be considered as the minimum requirements to achieve the desired effect.

Experience with Prospan LVL beams indicates that this degree of fixing may not satisfactorily prevent cupping of individual components as a result of the ingress of moisture between laminates during construction. The suggested method of vertical lamination below provides a greater level of fixity between individual components, and with the use of an elastomeric adhesive, also prevents moisture penetration between the laminates.

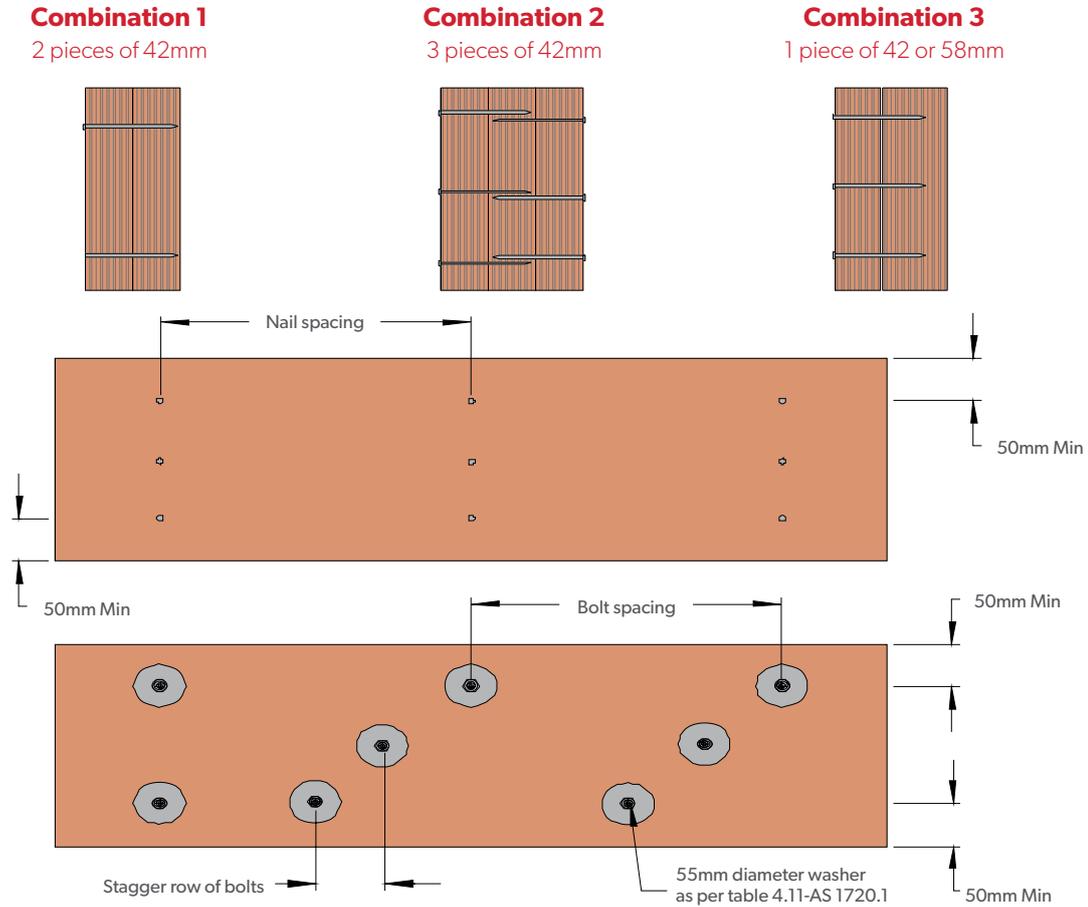
Multiple member laminating of top loaded beams (Symmetrical loading)

The edges of the individual sections must be carefully aligned to each other so that the composite beam is flat, allowing the applied loads to be equally shared.

- Depths up to and including 300 mm: 2 rows of nails as shown above at 300 mm centre
- Depths in excess of 300 mm: 3 rows of nails as shown above at 300 mm centres



Multiple Member Laminating of Side Loaded Beams (Non-symmetrical loading)



Maximum Floor Load Width Supported by Either Outside Member (mm)

Combination (see details above)	3.75Ø x 90mm nails		12mmØ bolts	
	2 rows at 300 ctrs	3 rows at 300 ctrs	2 rows at 600 ctrs	2 rows at 300 ctrs
Combination 1	3400	5100	7500	15000
Combination 2	2900	4000	5600	11000
Combination 3	2900	4000	4500	11000

Notes:

1. Table values are for 40 kg/m² floors.
2. The table values for nails may be doubled for nails at 150 mm centres, and tripled for nails at 100mm centres.
3. The nail schedules shown apply to both sides of a three (3) piece beam.
4. Bolts are to be grade 4.6 commercial bolts conforming to AS 1111. Bolt holes are to be a maximum of 13 mm diameter and are to be located NOT less than 50 mm from either edge.
5. All bolts shall be fitted with a washer at each end, of a size NOT less than that given in AS 1720.1 table 4.11.

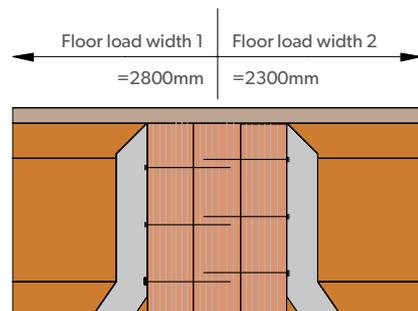
How to use the Maximum Uniform Side Load Table

Example: see diagram opposite

Beam of 2 Prospan LVL loaded on both side (Combination 1)

FLW 1 = 2800 mm, FLW 2 = 2300 mm Total FLW = 2800 + 2300 = 5100 mm.

1. Use SmartFrame software or SmartLVL safe load tables to size the two member section to support the FLW of 5100 mm.
2. Choose the larger of the side FLW's carried by the beam, in this case 2800mm.
3. Enter the table at the "Combination 1" row and scan across to a table value greater than 2800 mm. The first value in the row at 3600 mm is greater than the 2800 mm required.
4. Thus adopt 2 rows of 3.75Ø x 90 mm nails at 300 mm centres.



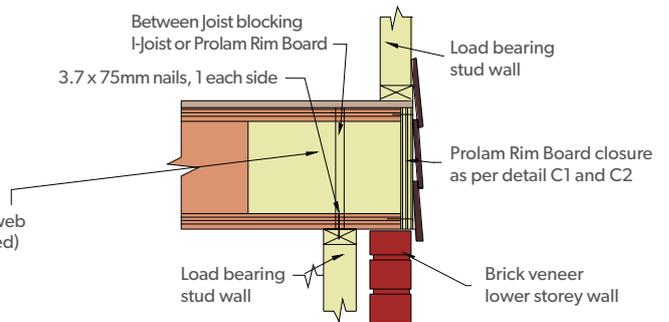
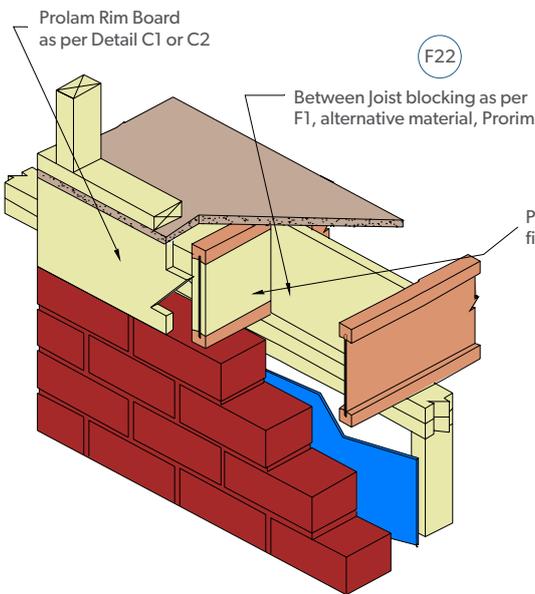
Brick Ledge Cantilevers Construction Details

Cantilevered Prolam I-joists as “brick ledge cantilevers” (Max of 160mm cantilever) to suit upper storey clad frames DO NOT usually require any special modification (other than the necessary timber or ply/LVL closure member attached to the outer edge as shown in details C1 or C2. The exceptions to this are where concentrated floor loads (e.g. truncated girders, jamb studs)

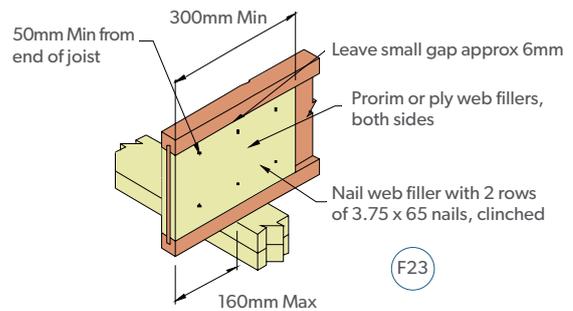
are supported on an Individual cantilevered joist such that the concentrated roof load area supported exceeds that as shown below for an un-reinforced Prolam I-joist.

Individual joists may be reinforced, if required, as per details F23 or F24 to support a roof load area (measured in square metres) as shown below.

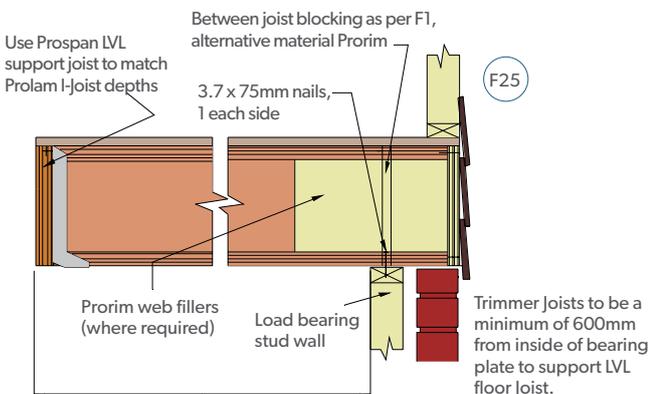
	Un-reinforced SmartJoist						Web Filler (F23)						Reinforcing one side (F24)					
	Sheet Roof			Tiled Roof			Sheet Roof			Tiled Roof			Sheet Roof			Tiled Roof		
Joist spacing	300	450	600	300	450	600	300	450	600	300	450	600	300	450	600	300	450	600
SmartJoist	Roof area supported (m ²)																	
PJ24070	14.2	13.2	12.2	8.3	7.6	6.9	15.6	14.5	13.3	9.1	8.5	7.8	17.6	16.4	15.2	10.3	9.6	8.9
PJ36090	17.8	16.1	14.4	10.4	9.4	8.5	21.2	19.6	17.9	12.4	11.4	10.5	19.3	17.6	15.9	11.3	10.3	9.3



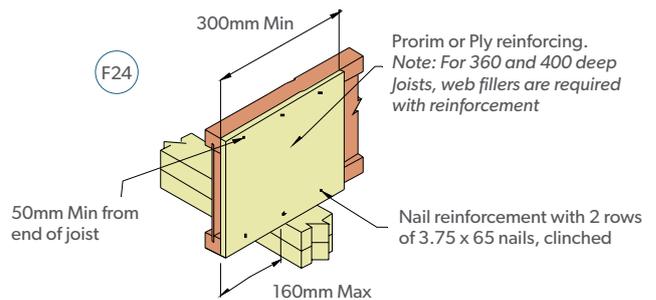
Reinforcing guidelines (a) using web fillers



Trimmer Joists where main joists run parallel to brick ledge cantilever wall



(b) Using reinforcing (one side)



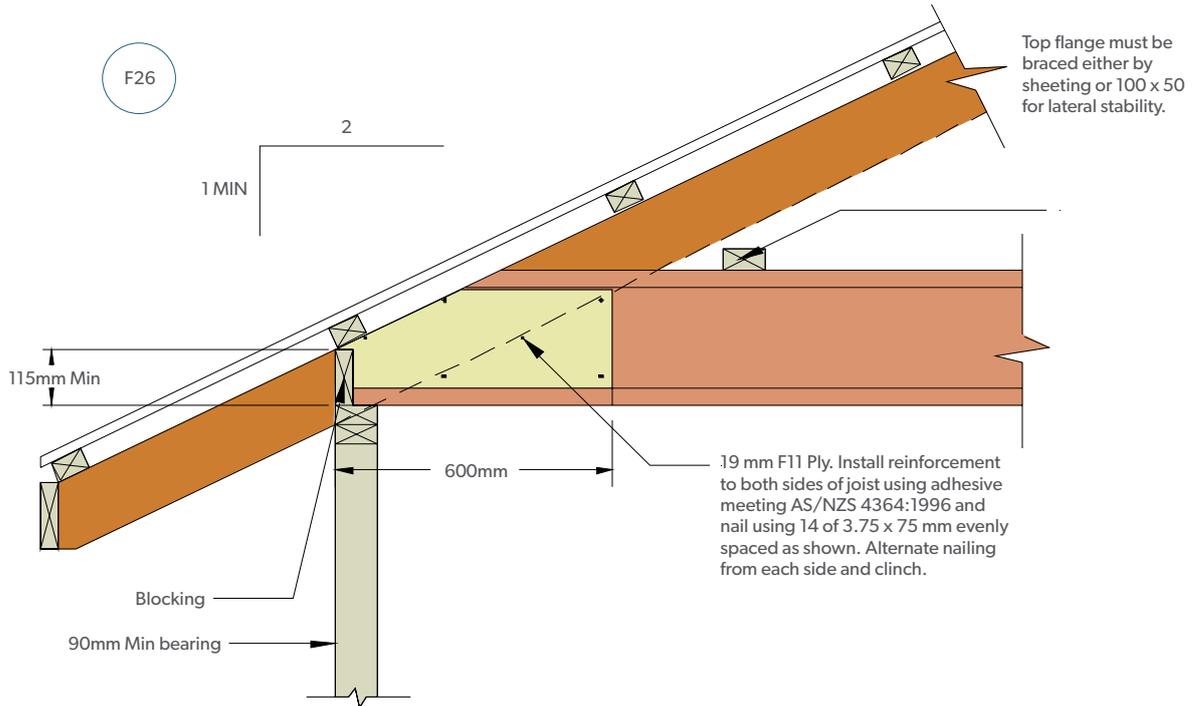
Rafter Cuts of Prolam I-joists

Prolam I-joists can be "rafter cut" but only within the limitation shown below.

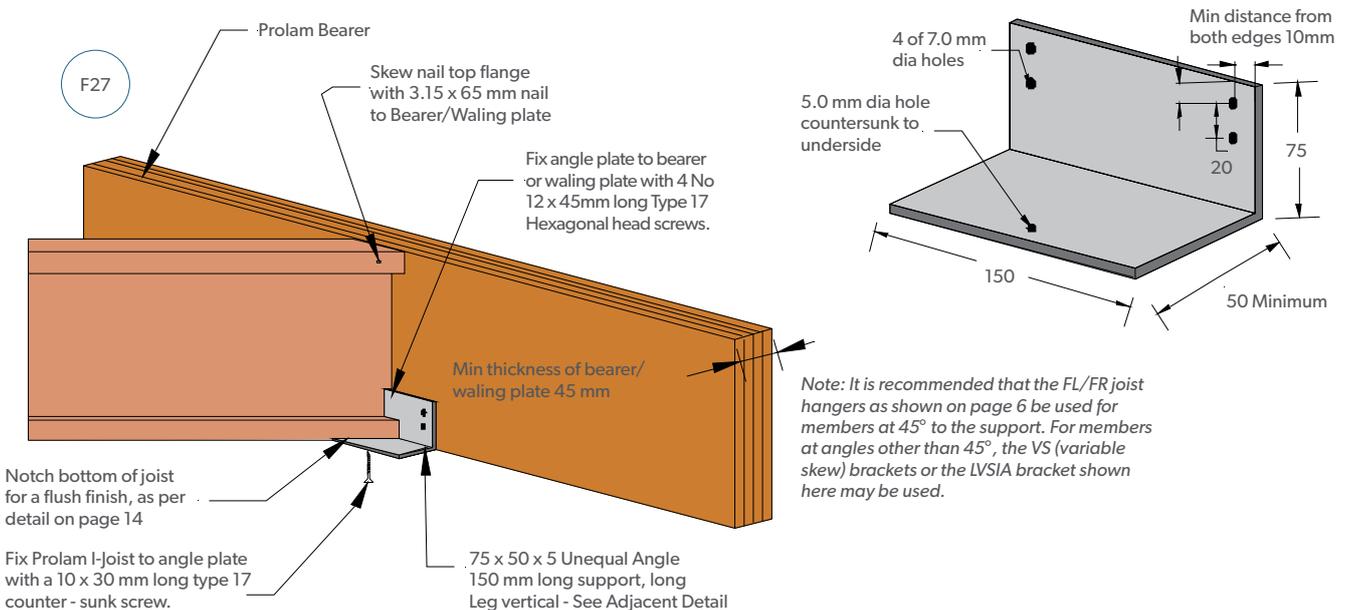
Rafter cuts are limited to:

1. 115 mm **minimum** end height
2. **Minimum** Roof Slopes of 1 in 2 (approximately 26.50), and
3. Must be blocked at the end to prevent rotation of the joist.

Joists without reinforcement are limited to design shear and end reactions up to 6.5 kN. Ply reinforcement can be added to joists with rafter cuts to increase the shear and end reaction capacity of the joist. The detail below shows the proper installation of the reinforcement. With the reinforcement added, the end reaction and shear capacity increase to 12.7 kN. Duration of load increases are permitted as per AS1720.1.

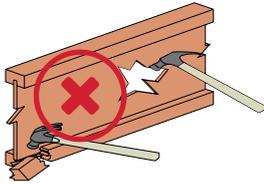


Oblique Connection Options

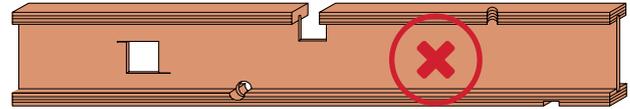


Prolam I-joists Hole Charts

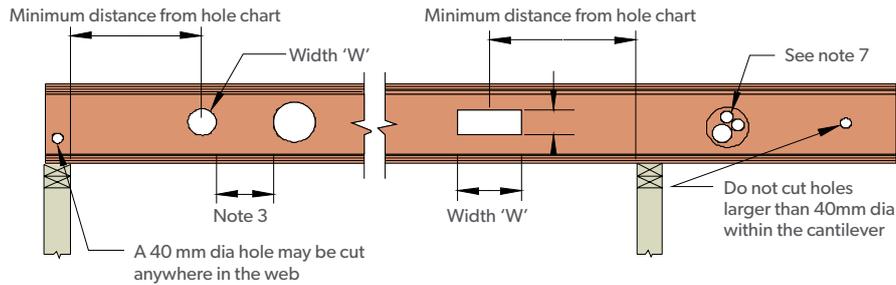
DON'T make holes with hammer other than pre-punched knockouts



DON'T hammer on flanges and damage joint



**DO NOT cut or notch flanges
DO NOT over cut holes in web**



Note: The most accurate method to design the allowable web penetration size and distance from support for Prolam I-joists is to use the Prolam Online software. The table below will give conservative results in some instances.

Joist Code	Joist span (mm)	Joist spacing (mm)	Circular Holes								Rectangular Holes			
			Hole Diameter / Square Hole Width (mm)								Depth x Width (mm)			
			75	100	125	150	175	200	225	250	125x150	150x300	175x350	200x400
Minimum distance from any support to the centre of the hole (mm)														
PJ24070	3500	600	300	300	300	1100	-	-	-	-	1450	1750	-	-
	4000	600	300	300	300	1400	-	-	-	-	1800	2000	-	-
	4500	600	300	300	600	1600	-	-	-	-	2250	2250	-	-
PJ30070	4500	600	300	300	300	300	450	1400	-	-	1550	2100	2200	2250
	5000	600	300	300	300	300	700	1650	-	-	2000	2400	2500	2500
PJ36090	5500	600	300	300	300	300	300	300	700	1600	400	1950	2450	2650
	5800	600	300	300	300	300	300	300	850	1750	700	2150	2650	-

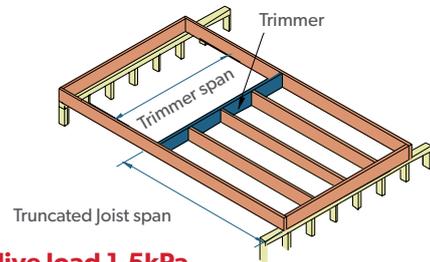
Notes:

- The hole chart is generated on a maximum floor dead load of 40kg/m with no wall or roof loads. It therefore does not apply for joists supporting either parallel or perpendicular load bearing walls.
- Hole locations are suitable for joist spacings up to 600mm centres. Holes may be permitted closer to supports for some member when spacings of 450 or 300mm are used.
- The clear distance between holes must equal or exceed twice the diameter of the largest hole, or twice the longest side of a rectangular hole and no more than 3 holes in excess of 75mm are allowed in any span.
- Do not cut or damage flanges under any circumstances.
- Except as noted in 1 and 2 above, a 40mm hole at a minimum of 450mm centres is allowed to be drilled anywhere in the web except in cantilevered spans.
- If possible, holes in web should be positioned mid height, minimum edge clearance from any flange is 6mm.
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

Opening within Prolam I-joists Floors

Opening Trimmer

The tables below are for trimmer members of Prolam I-joists and LVL. Other Prolam engineered timber products may also be used for this member, the designs for each of these other material types can be simply calculated by using the Prolam Online software.



Floor live load 1.5kPa

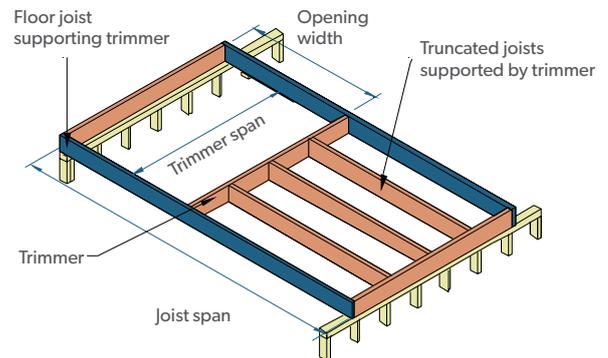
Prolam I-joist	Maximum trimmer span (mm)				
	Truncated joist span (mm)				
	1.5	3.0	4.5	6.0	7.5
PJ24070	4600	3800	3400	3000	2800
PJ30070	5200	4300	3800	3500	3300
PJ36090	6100	5000	4500	4100	3900

Floor live load 1.5kPa

Prolam LVL 15	Maximum trimmer span (mm)				
	Truncated joist span (mm)				
	1.5	3.0	4.5	6.0	7.2
190 x 42	3900	3200	2800	2500	2300
240 x 42	4500	3800	3300	3000	2800
290 x 42	5300	4500	4000	3700	3500

Trimmer Joists

Trimming joists at floor opening often support the loads from stair stringers, as well as the concentrated load from the trimmer. The table below has been set up to allow a load from stairs equally supported from the floor below (or above) and the trimmer joists. In many cases this will provide a conservative result.



Floor live load 1.5kPa

Prolam engineered timber		Maximum trimming joist span (m)											
		Opening width 900 (mm)				Opening width 1800 (mm)				Opening width 2700 (mm)			
		Trimmer span (mm)				Trimmer span (mm)				Trimmer span (mm)			
		2	3	4	5	2	3	4	5	2	3	4	5
Prolam I-joist	PJ24070	4200	3900	3700	3500	4100	3800	3600	3500	4400	4200	4000	4000
	PJ30070	4900	4500	4200	4000	4600	4300	4100	3900	4800	4600	4400	4300
	PJ36090	5700	5300	5000	4800	5400	5000	4800	4400	5500	5100	4900	4700
Prolam LVL 15	200 x 42	3600	3200	2900	2700	3600	3300	3100	3000	4000	3900	3700	3600
	240 x 42	4200	3800	3500	3300	4100	3800	3600	3400	4400	4200	4000	3900
	300 x 42	5100	4600	4300	4100	4800	4400	4100	4000	5000	4700	4500	4300

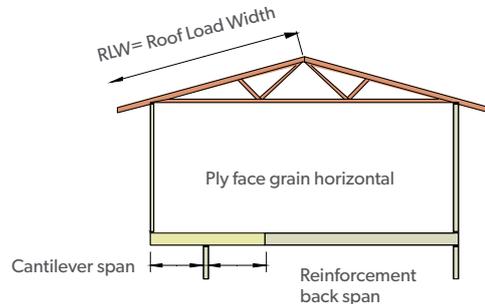
Note: Based on spacing of 600 mm and maximum of 10 mm DL deflection

Prolam I-Joist Cantilevers Supporting Load Bearing Walls

Prolam I-joist cantilevers may need to be reinforced to support load bearing walls at the end of the cantilever. The table below lists the allowable roof load widths with un-reinforced and reinforced Prolam I-joists.

Reinforcement Description:

- 0. Reinforcement not required
- 1. Install 15 mm F11 x 1200 mm min structural ply on one side of joist
- 2. Install 15 mm F11 x 1200 mm min structural ply on both sides of joist or double joist at cantilever
- x. Not suitable



Tables assume a 0.8 kN/m wall dead load, sheet roof dead load of 40 kg/m² and tiled roof dead load of 90 kg/m².
 Serviceability Limits on Cantilever - DL: 6.0 mm Max
 LL: 4.5 mm Max

Reinforcement requirements for cantilevered floor joists supporting load bearing walls*

***Important:** See notes at bottom of page on the use of this table

Note: Total length cantilever reinforcement must be a minimum of 1200 mm but NEVER be less than twice the cantilever span. i.e. Reinforcement back span ≥ cantilever span.

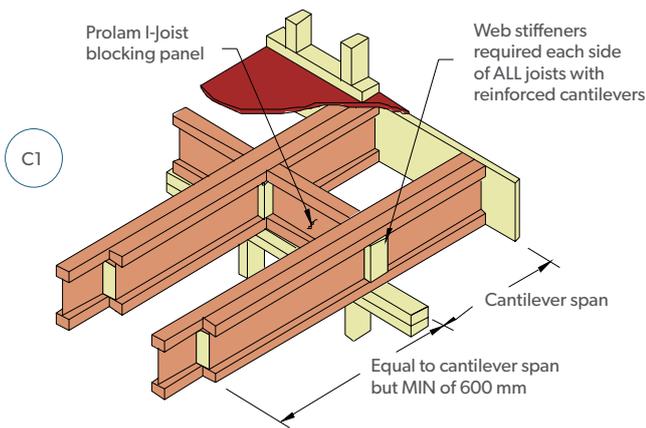
Max cantilever (mm)	RLW (m)	Sheet Roof 40kg/m ²									Tiled Roof 90kg/m ²														
		4.0			6.0			8.0			2.0			4.0			6.0			8.0					
		300	450	600	300	450	600	300	450	600	300	450	600	300	450	600	300	450	600	300	450	600			
300	PJ24070	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PJ30070	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PJ36090	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
600	PJ24070	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	x
	PJ30070	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PJ36090	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
900	PJ24070	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	x	x	x	x	x
	PJ30070	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2	0
	PJ36090	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1200	PJ24070	0	0	0	0	0	0	x	x	x	0	0	x	x	x	x	x	x	x	x	x	x	x	x	x
	PJ30070	0	0	0	0	0	0	0	0	0	0	0	0	0	1	x	x	x	x	x	x	x	x	x	x
	PJ36090	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	x	0

Spans in the preceding table are based upon a uniform roof load width at the cantilever end of each joist. The presence of large windows or openings within the load bearing wall supported by these cantilevered joists create concentrated loads at the edges of such openings. The joists supporting the concentrated loads will require special engineering consideration to avoid excess loads and differential deflections between adjacent joists. It is strongly recommended that where concentrated loads occur on cantilevered joists that advice be sought from the SmartData Customer Helpline on 1300 668 690 or at smartdata@tilling.com.au.

Example Construction Details for Load-Bearing Cantilevers

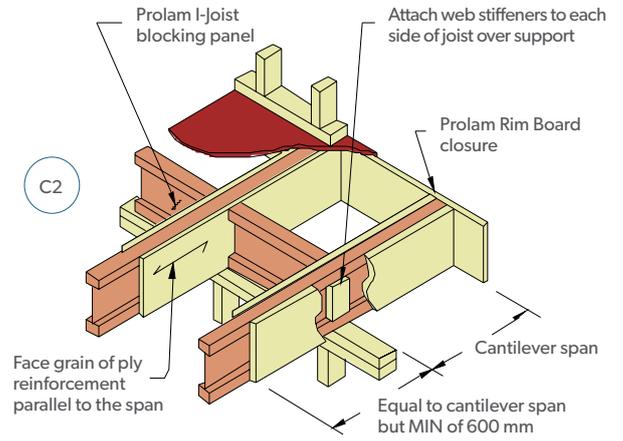
Note: Option 1 with cantilever reinforced with an extra Prolam I-joist is equivalent to option 2 with 2 sheets of ply reinforcement.

Option 1 Cantilever Reinforced with Extra Prolam I-joist



Note: Block together full length with filler blocks as per detail F15 of the Prolam I-joist Design Guide.

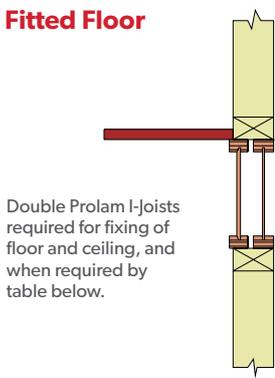
Option 2 Cantilever Reinforced with 1 or 2 sheets of reinforcing ply



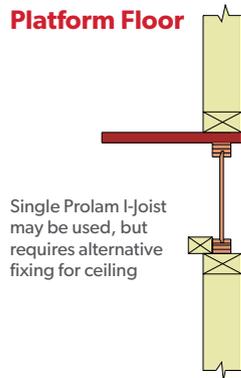
Note: 15 mm F11 structural ply is required on one (P1) or both sides (P2) of the joist. (See Tables). Depth shall match the full height of the Prolam I-joist. Nail with 3.15 x 65 Nails at 100 mm ctrs in a staggered pattern.

Smart Joists Supporting Parallel Load Bearing Walls

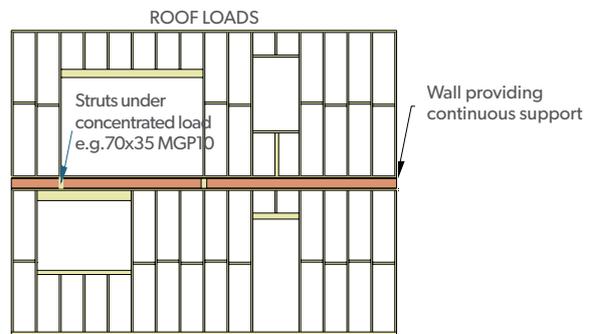
Fitted Floor



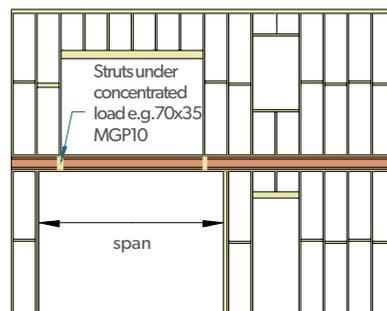
Platform Floor



Joists Continuously Supported by Wall



Joists not continuously supported by wall



Single (and Double) Prolam I-joists are adequate to transfer uniformly distributed compression loads up to 29 kN/m per joist from load bearing walls to a continuous rigid support below. Detail F5 is to be used where concentrated loads are to be transmitted through the Prolam I-joist floor system.

The table gives allowable spans for single or double floor joists NOT continuously supported by a parallel wall under. Care must be taken to adequately support the web of the joists from concentrated span point loads, by the use of detail F5.

Single Prolam I-Joists Supporting Parallel Load Bearing Walls

Floor Load Width (mm)		900			1200			1500		
Roof Load Width (mm)	Roof mass (kg/m ²)	1500	3000	5000	1500	3000	5000	1500	3000	5000
Prolam I-Joist		Maximum Single span (mm)								
PJ24070	40	3500	3100	2900	3300	3000	2700	3100	2900	2600
	90	3100	2600	2200	3000	2500	2100	2900	2400	2000
PJ30070	40	4000	3600	3300	3800	3500	3200	3600	3300	3100
	90	3500	3100	2700	3400	3000	2600	3300	2900	2500
PJ36090	40	4700	4200	3900	4400	4100	3700	4200	3900	3600
	90	4200	3600	3200	4000	3500	3100	3900	3400	3100
Maximum Continuous span (mm)										
PJ24070	40	4500	4100	3800	4300	3900	3600	4100	3800	3500
	90	4000	3500	2500	3900	3200	2400	3700	3000	2300
PJ30070	40	5100	4700	4300	4900	4500	4200	4700	4300	4000
	90	4600	4000	3200	4400	3800	3000	4300	3700	2900
PJ36090	40	6000	5500	5000	5700	5300	4900	5500	5100	4700
	90	5400	4700	3900	5200	4600	3800	5000	4500	3600

Notes:

1. Bearing lengths - minimum of 30 mm
2. Wall loads assumed 0.37 kPa
3. Floor load loads 1.5 kPa or 1.8 kN point live load.
4. Upper floor dead load 40 kg/m²
5. Deflection limits: permanent load - span/300 or 12 mm max LL - span/360 or 9 mm max.

Double Prolam I-Joists Supporting Parallel Load Bearing Walls

Floor Load Width (mm)		900			1200			1500		
Roof Load Width (mm)	Roof mass (kg/m ²)	1500	3000	5000	1500	3000	5000	1500	3000	5000
Prolam I-Joist		Maximum Single span (mm)								
2/PJ24070	40	4300	4000	3600	4100	3800	3500	3900	3700	3400
	90	3900	3400	3000	3700	3300	3000	3600	3300	2900
2/PJ30070	40	4900	4500	4100	4700	4300	4000	4500	4200	3900
	90	4400	3900	3500	4300	3800	3400	4100	3700	3400
2/PJ36090	40	5800	5300	4900	5500	5100	4700	5300	4900	4600
	90	5200	4600	4100	5000	4500	4000	4800	4400	3900
		Maximum Continuous span (mm)								
2/PJ24070	40	5400	5000	4600	5200	4800	4400	5000	4600	4300
	90	4900	4300	3800	4700	4200	3700	4600	4100	3600
2/PJ30070	40	6200	5700	5000	5900	5400	4700	5500	5100	4600
	90	5500	4500	3800	5200	4400	3700	5000	4200	3600
2/PJ36090	40	7300	6700	6100	6900	6400	6000	6600	6200	5700
	90	6600	5700	4800	6300	5500	4700	6100	5300	4600

Notes:

1. Bearing lengths - minimum of 30 mm
2. Wall loads assumed 0.37 kPa
3. Floor load Loads 1.5 kPa or 1.8 kN point live load.
4. Upper floor dead load 40 kg/m²
5. Deflection limits: permanent load - span/300 or 12 mm max LL - span/360 or 9 mm max.

Prolam Rim Board Hole Specifications

The maximum allowable hole size for a SmartRim shall be $\frac{2}{3}$ of the Rim Board depth as shown below.

The length of the Rim Board segment containing a hole shall be at least 8 times the hole size.

Rim Board Hole Sizes and Minimum Length

Prolam Rim Board Depth (mm)	Maximum allowable hole size ^{(a)(b)} (mm)	Minimum length of Prolam Rim Board segment ^(c) for the maximum allowable hole size (mm)
200	130	1050
240	160	1280
300	200	1600
360	235	1900

(a) These hole provisions do not apply to Prorim installed over openings such as doors or windows

(b) The diameter of the round hole or the longer dimension of the rectangular hole

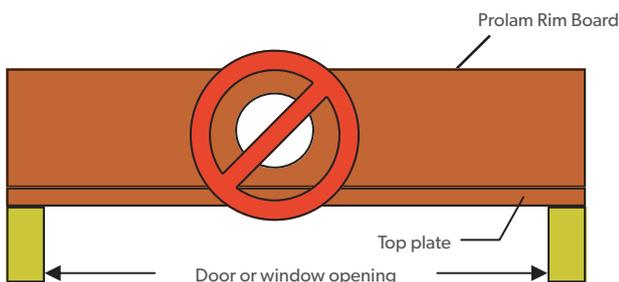
(c) The lengths of the Prorim segment per wall line. For multiple holes, the minimum length of Prorim segment shall be 8 times the sum of all hole sizes

Application Notes

- Do not cut holes in Prorim installed over openings, such as doors or windows, where the Prorim is not fully supported, except that holes of 40 mm or less in size are permitted provided they are positioned at the middle depth and in the middle $\frac{1}{3}$ of the span (see note 5 for minimum hole spacing).
- Field-cut holes should be vertically centred in the Rim Board and at least one hole diameter or 150 mm whichever is less, clear distance away from the end of the wall line. Holes should never be placed such that they interfere with the attachment of the Rim Board to the ends of the floor joist, or any other code-required nailing.

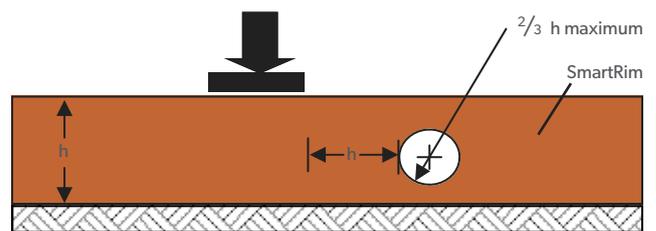
Rim board over an opening

Do not cut holes in Rim Board over an opening except for holes of 40mm or less in size (see note 1)



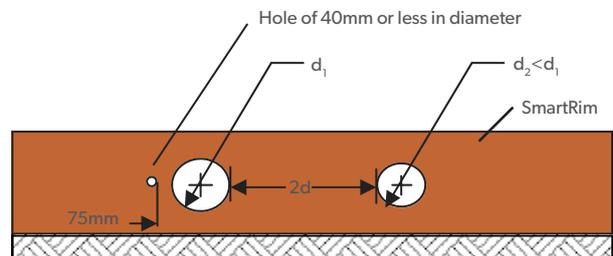
- While round holes are preferred, rectangular holes may be used providing the corners are not over-cut. Slightly rounding corners or pre-drilled corners with a 25 mm diameter bit is recommended.
- When concentrated loads are present on the Rim Board (loads not supported by any other vertical-load-carrying members such as squash blocks), holes should not be placed in the Rim Board within a distance equal to the depth of the Rim Board from the area of loading.

Rim board near concentrated vertical load



- For multiple holes, the clear spacing between holes shall be at least two times the diameter of the larger hole, or twice the length of the longest rectangular hole. This minimum hole spacing does not apply to holes of 40 mm or less in diameter, which can be placed anywhere in the Rim Board (see note 1 for holes over opening) except that the clear distance to the adjacent hole shall be 75 mm minimum.

Multiple Holes for Rim Board



- All holes shall be cut in a workman-like manner in accordance with the limitations listed above.

Prolam I-Joist Roof Details

About Roofs

Roof members are subject to dead and live loads as well as wind loads. These wind loads can act either down onto the roof, or can create an uplift effect. For roofs of light construction, the uplift loads generally control the maximum span, whereas it is usual for dead and live loads to be the controlling factors for heavier roofs (e.g. tiles). Prolam I-joists, by their large depth to width ratio, perform well in roof situations providing that their upper and lower flanges have adequate lateral support provided by battens and/or ceiling materials. Due to this fact, the spans in the table below only apply for roofs which meet the following criteria:

- Enclosed building

- Ceiling fastened to the underside of bottom flange on adequate lateral supports to bottom flange at a minimum of 600 mm centres
- Roofs are constructed as per details R1 to R9 of this manual
- Batten spacings at a maximum of 1200 centres.

Tie Down

Wind loadings on light roofs can produce net uplift pressures. The same requirements and methods of tie down apply to Prolam I-joists as for solid timber roof members except that any tie down system must extend over the top flange. Guidance for tie down requirements are provided in AS1684.

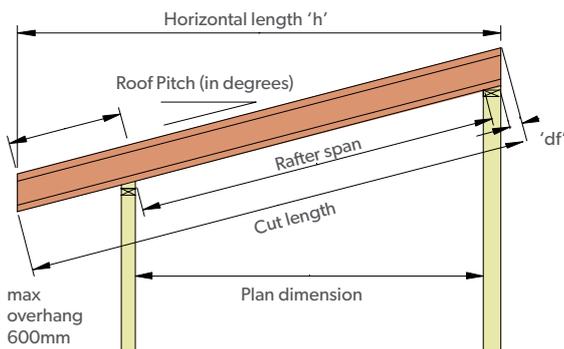
Wind Classification N1-N3

Max Deflections DL: Span/300 LL: Span/250
 WL: Span/150. Max Slope = 25°

Joist Code	Roof mass (kg/m ²)	Single span at 25° pitch				Continuous 2 span at 25° pitch			
		Rafter/Roof Beam spacing (mm)							
		450	600	900	1200	450	600	900	1200
Recommended maximum rafter span - Plan dimension (mm)									
PJ24070	40	6600	6050	5350	4850	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾
	90	5100	4650	4050	3650	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾
PJ30070	40	7800	7200	6350	5800	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾
	90	6100	5550	4850	4400	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾
PJ36090	40	9150	8650	7800	7150	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾
	90	7550	6900	6000	5450	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾	N/A ⁽¹⁾

Note: ⁽¹⁾ Maximum Continuous spans exceed the maximum available length of the Prolam I-joist

Sloped Roof Span and Cut Lengths



Roof slope degrees	Slope factor 'sf'	Depth factor 'df'				
		Joist depth (mm)				
		200	240	300	360	400
15	1.04	54	64	80	96	107
17.5	1.05	63	76	95	113	126
20	1.06	73	88	109	131	146
22.5	1.08	83	99	124	149	166
25	1.10	93	112	140	168	187
27.5	1.13	104	125	156	187	208
30	1.15	115	139	173	208	231
35	1.22	140	168	210	252	280

span (mm) = plan dimension x slope factor (sf)
 Cut length (mm) = horizontal length (h) x slope factor (sf) + depth factor (df) = h x sf + df

Typical Prolam I-Joist Roof Details

WARNING: Do not allow workers or loads on roof until ALL blocking, hangers, bracing and nailing is completed. SEE SAFETY WARNING.

Birdsmouth Cut

(At low end of joist ONLY)
(Limited to joist spacing of MAX of 600 mm)

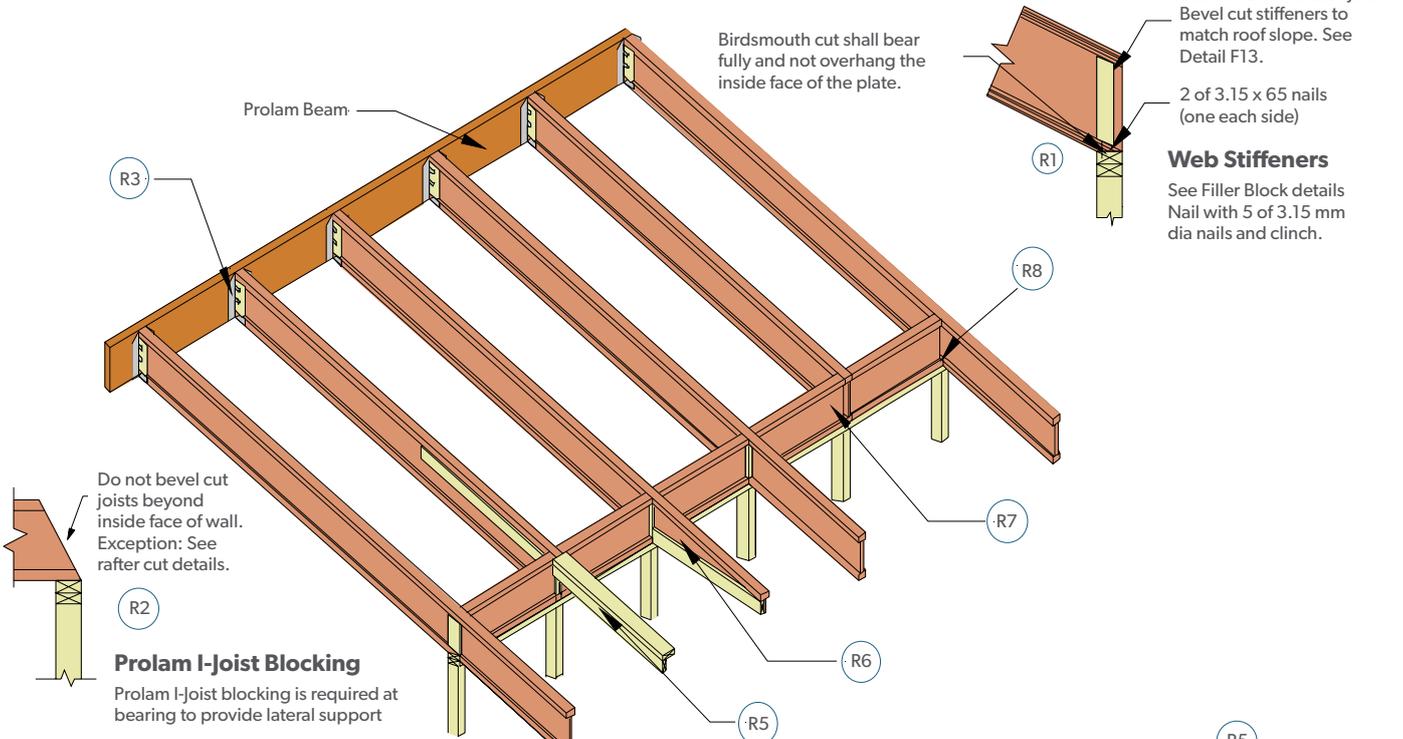
Birdsmouth cut shall bear fully and not overhang the inside face of the plate.

Web stiffeners required each side of Prolam I-joist. Bevel cut stiffeners to match roof slope. See Detail F13.

2 of 3.15 x 65 nails (one each side)

Web Stiffeners

See Filler Block details
Nail with 5 of 3.15 mm dia nails and clinch.



Do not bevel cut joists beyond inside face of wall. Exception: See rafter cut details.

R2

Prolam I-joist Blocking

Prolam I-joist blocking is required at bearing to provide lateral support

R3

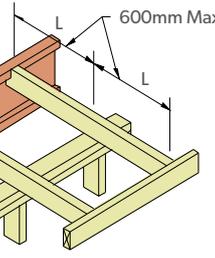
Supported beam

Bevelled web stiffener each side

Use LFVS rafter hanger or similar

R4

Joist shall be designed using design properties when "L" exceeds joist spacing.

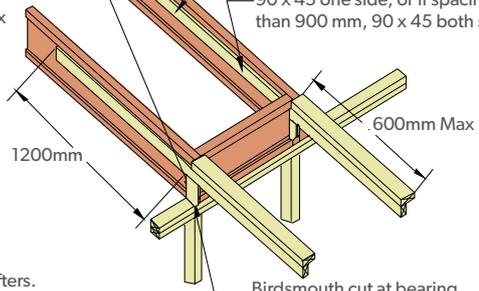


50mm width overhang rafters. Notch around Prolam I-joist top flange.

50mm width cripple, cut under 90 x 45 rafter extension (Web stiffener other side)

Use 2 rows of 3.15 x 65 nails at 200 mm centres

90 x 45 one side, or if spacing is greater than 900 mm, 90 x 45 both sides.

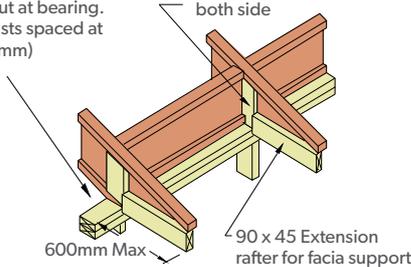


Birdsmouth cut at bearing. (Birdsmouth cuts limited to joist spacing of Max 600 mm)

R6

Birdsmouth cut at bearing. (Limited to joists spaced at a Max of 600mm)

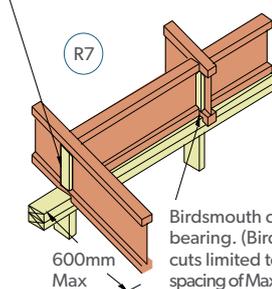
Web stiffener required both side



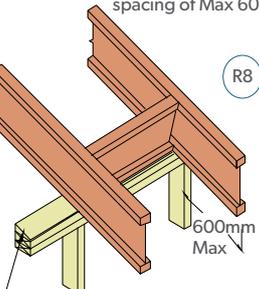
R7

Web stiffener required

Birdsmouth cut at bearing. (Birdsmouth cuts limited to joist spacing of Max 600mm)



R8

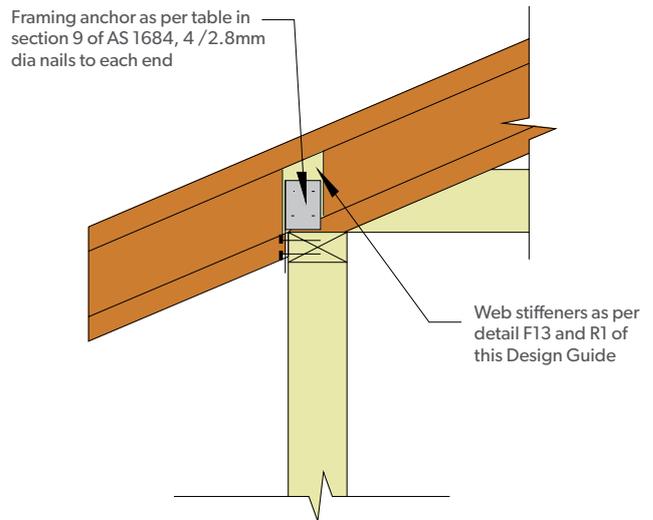
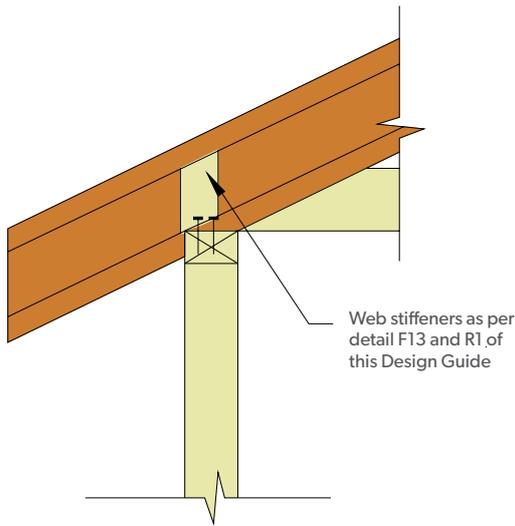


50mm beveled plate for slopes greater than 1 degree. ALTERNATIVE: Use birdsmouth details R1 and R7.

Prolam I-Joist Rafter Tie Down

Prolam I-joist rafters need to be tied down in wind uplift situations in a similar manner to solid timber as shown in section 9 of AS 1684. The examples shown in this section are equally applicable to Prolam I-joist except that web stiffeners as per detail F13 and R1 must be installed to the Prolam I-joist where either skewed nails or framing anchors are chosen as the tie down method before the uplift capacities in the tables in section 9 of AS 1684 can be adopted.

All tie down types that involve a strap over the top of the Prolam I-joist rafters, or involving the bolting down of a member above the rafter running in the perpendicular direction, require no modification to the Prolam I-joist and the uplift capacities in the tables in section 9 of AS 1684 may be used.



Typical Prolam I-Joists Rafter Box Gutter Rebate Details

Box Gutter Rebates

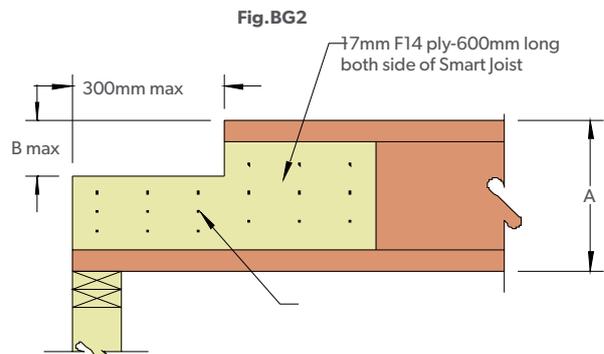
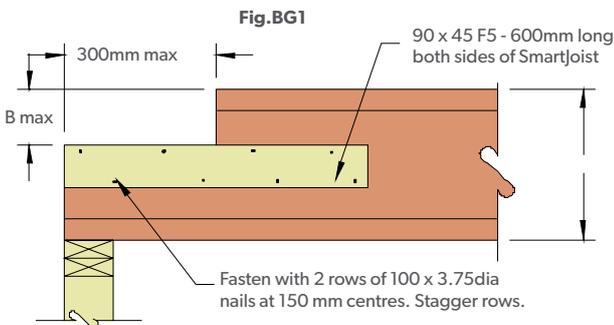
Rebates for box gutters are permissible within a roof constructed with Prolam I-joist rafters to the maximum rebate limits as shown below.

Fig.BG1 with 2 pieces of 90 x 45 nailed to the web reduces shear capacity by 40%

Fig.BG2 with 2 pieces of 17 mm F14 ply nailed to the web maintains full shear capacity

Given that the design shear values at the end of rafters with lightweight roofs are usually very low compared to the allowable shear, in most instances fig BG1 is satisfactory to provide a box gutter rebate within the Prolam I-joist rafters, however the remaining shear capacity must be checked.

Prolam I-joist box gutter rebate details



A = 200*, 240 & 300 mm depth
 B = 50 mm when A = 200* & 240 mm
 B = 100 mm when A = 300 mm

*200 mm - Requires ply infill, 90 x 45 solid timber reinforcement is not suitable

General Connector Installation Details

Positive Angle Nailing

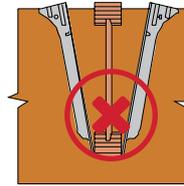


Correct Nailing

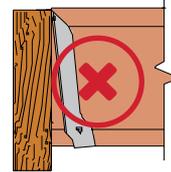
Nail at wrong angle

Nail too long

Top Mount Hangers



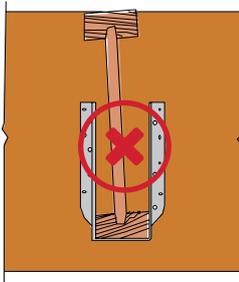
Hanger over spread
If hanger is overspread, I-joist may be raised above header, also, NO support for top flange



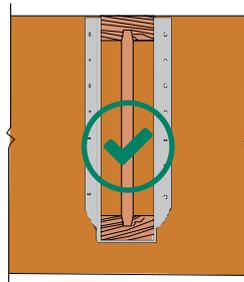
Hanger not plumb
A hanger kicked out from the header can cause uneven surfaces

Prevent Rotation

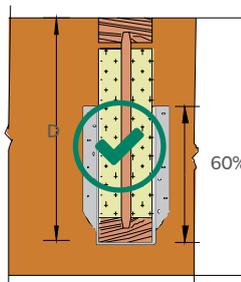
Hangers provide some joist rotation resistance; however, additional lateral restraint may be required for deep joists.



No web resistance results in rotation



No web stiffener required
Hanger side flange supports joist top flange.



Web Stiffener required
Hanger side flange should be at least 60% of joist depth or potential joist rotation must be addressed

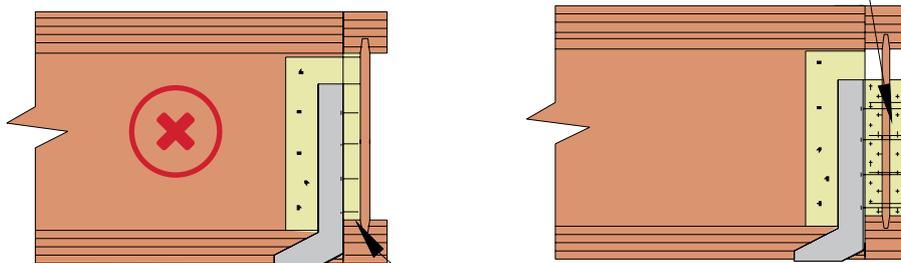
Correct Fasteners



Bracket capacities are based upon using the correct bracket nail as per the table on page 6. Bracket nails have special heads to provide strength. Clouts, brads etc are **not** suitable as bracket nails.

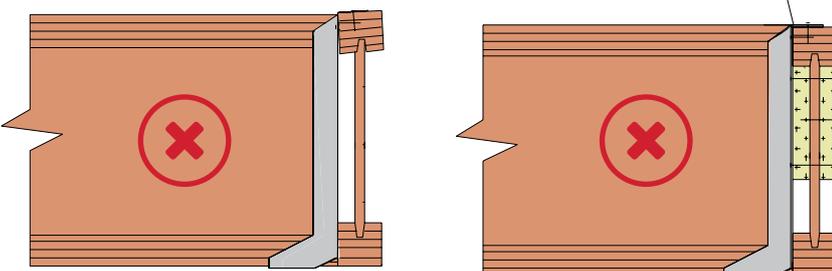
I-joist Headers

Backer blocking each side, hanger nails must extend past the supporting joist's web member into the backer blocking



Bottom flange pulling off when Backer block on one side only

The top flange of the supporting joist must be supported by backer blocks to prevent cross grain bending and rotation.



Face Mount Connection to web

Top Mount Connection

Fire Safety and Sound Transmission

Fire Safety

The Building code of Australia became a performance based code in 1996 (BCA96). The introduction of the BCA along with the changes to AS1530.4 has seen Australia brought into line with international standards for fire resistance testing. The principle modification has been to express the test result in terms of the performance of the specimen Fire Resistance Level (FRL) rather than to assign a single rating as had been the established practice. The outcome of the test is expressed as the number of minutes for which the specimen fulfils the requirements of each of the three criteria, being:

- a) Structural adequacy
- b) Integrity; and
- c) Insulation, and expressed in that order

The performance of a specimen is then given as the actual time for which the specimen satisfied these criteria, but rounded down to the nearest regulatory requirements. e.g. 60/60/60.

Ceiling systems may also be required to provide "Resistance to the Incipient Spread of fire" for a given period of time. This requires the ceiling system to prevent the spread of fire within a roof/ceiling or floor/ceiling cavity by providing adequate thermal insulation to combustibles in this area, avoiding the danger of them igniting. To coincide with the changes, the timber industry produced a set of manuals Multi Residential Timber Framed Construction (MRTFC) 1, 2 & 3, which outlined the BCA requirements for multi residential buildings, design criteria and construction details which were designed to satisfy the BCA requirements.

Fire Rated Floors/Ceilings

The best information available at this time concludes that the fire resistance for ceiling and floor/ceilings is achieved by the lining material and that alternative joist sizes and material can be substituted in the various certified systems, providing they are designed to support the full loads. The thickness of the fire grade ceiling lining for the relevant FRL is the same as required in the tested systems using solid timber joists.

Additional testing have concluded that the following layers of fire rated plasterboard can achieve the FRL and incipient spread of fire as listed in the following table:

Fire grade plasterboard	FRL	Incipient spread of fire
1 x 13mm	30/30/30	0
1 x 16mm	60/60/60	30
2 x 13mm	60/60/60	30
2 x 16mm	90/90/90	60
3 x 16mm	120/120/120	60

The above ratings can be achieved using standard fire grade plasterboard from some sheet manufacturers. Other manufacturers may however require the use of special fire rated board or may still require the number of layers outlined in MRTFC 2.

For further details on the various certified systems, see the MRTFC manuals available through all State Timber Development Associations or contact the engineers on the SmartData Customer HelpLine on 1300 668 690.

Sound Transmission

The ability of walls and floors to reduce noise is measured over the most important part of the hearing range (from 125 to 4000 cycles per second), and the results reduced to a "weighted sound reduction index" or Rw value. In 2004, the BCA introduced the addition of a Spectrum Adaption factor. This Ctr factor takes into account lower frequency level sounds, and has been chosen in large part, in recognition of the problem of the high bass frequency outputs of modern home theatre systems and amplified music systems. Therefore, both the Ctr and the Rw of the building element will now need to be considered.

In addition to being rated for airborne sound transmission, floors are also rated by "Impact sound pressure level" or L'n,w plus the spectrum adaption factor CI values that rate the capacity of floor assemblies to control impact noise such as footfalls. The lower the L'n,w +CI of the floor, the better the performance of the floor in terms of impact sound insulation.

The BCA now requires a Rw+Ctr of 50 in floors between sole occupancy units and between dwellings and a plant room, lift shaft, stairway, public corridor, public lobby or similar.

In 2004, the BCA introduced Deemed-to-satisfy provisions which require the L'n,w +CI of a floor to be determined by testing in the laboratory. The impact sound insulation requirements for floors in the BCA is L'n,w +CI not more than 62 for floor separating dwellings and floor separating dwellings from a plant room, lift shaft, stairway, public corridor, public lobby or similar.

The use of light-frame construction systems challenges designers to insulate against noise rather than simply relying on the massiveness of heavy walls and floors. Excellent levels of noise control can be achieved with good acoustics in wood framed structures surfaced with wood structural panels. Sound control can be achieved by applying floor and wall materials over isolated air spaces that absorb sound. The addition of resilient channels to support the ceiling system independently increases the Rw+Ctr and L'n,w +CI ratings.

The best current understanding indicates that the "Certified Systems – Walls, Floors and Ceiling" as detailed in the MRTFC 2 can be used to closely approximate the Rw+Ctr and L'n,w +CI rating of floor/ceiling systems with SmartJoist floor joists. Work is under way to further investigate the link between joist types and impact sound insulation.

For further details on the various certified systems, see the MRTFC manuals available through all State Timber Development Associations or contact the engineers on the SmartData Customer HelpLine on 1300 668 690 or at smartdata@tilling.com.au

The treatment of timber to extend its service life is covered by AS/NZS 1604. This code identifies the various biological hazards by a hazard class number, with hazard class numbers from H1 - H6. The higher the hazard class number, the greater the severity of the biological hazard. A complete table listing the 6 hazard classes is contained within AS/NZS 1604.1.

All are supplied treated to either H1.2. It is also NOT recommended that SmartJoists be specified for use outside above ground. This is predominately due to the geometric shape of the I-joist which will not shed water effectively.

*All Prolam I-joists are manufactured from materials not susceptible to Lyctid attack

Adhesive and Formaldehyde Emission Facts

Q. Are the glues used in SmartFrame Engineered Wood Products safe?

- A. Yes, they are safe, Phenolic resins used in our Engineered Wood Products are stable, polymerised materials. The polymerisation reaction is non-reversible (i.e. once the polymer is formed, it doesn't break down). A wood dust warning label is provided for all SmartFrame wood products to alert our customers that wood dust can be generated by sawing, sanding, or machining wood and wood products.

Q. What is the level of formaldehyde emission from our phenolic-bonded Engineered Wood Products?

- A. Independent third party testing has confirmed that formaldehyde emissions from our phenolic-bonded products (i.e. OSB, LVL, I-Joists, and Glulam) are below 0.5mg/L under reasonably foreseeable conditions of use, which meets or exceeds the E0 Formaldehyde Emission Class.

In short, all available information indicates that formaldehyde levels associated with phenolic resin-bonded wood products are similar to those of the dimension lumber veneer or other forms of wood used to make the products.

Q. How much formaldehyde is in our phenolic-bonded, Engineered Wood Products?

- A. The amount of formaldehyde in our Engineered Wood Products is less than 0.1 percent of the dry weight.

Q. What is being done to reduce the exposure to formaldehyde?

- A. Formaldehyde is normally present at low levels, usually lower than 0.03 ppm, in both outdoor and indoor air. Efforts have been made by both government and industry to reduce exposure to formaldehyde. A 1985 regulation by the US Department of Housing and Urban Development (HUD), covering the use of manufactured pressed wood products in housing was designed to ensure that indoor levels were below 0.4 ppm. Product standards established for plywood and particleboard led to significant reductions in formaldehyde emissions from those products. Furthermore, HUD acknowledged that phenolic resin bonded wood products emitted such small quantities of formaldehyde that these products were exempted from all the testing and certification requirements of the standards. In Germany, the German Hazardous Materials regulation, better known as the "E1" Standard, sets a limit of 1.0 mg/L for formaldehyde emissions from some wood-based composite products. All available data indicates that our phenolic bonded Engineered Wood Products meet the more stringent E0 level.

Q. What affects formaldehyde levels in a home?

- A. Formaldehyde levels in the indoor air depend mainly on what is releasing the formaldehyde, the temperature, the humidity, and the air exchange rate (i.e. the amount of outdoor air entering or leaving the indoor area) Levels of formaldehyde decrease with increasing air exchange rate, decreasing temperature, and decreasing humidity.